831

See Sheet 1A For Index of Sheets

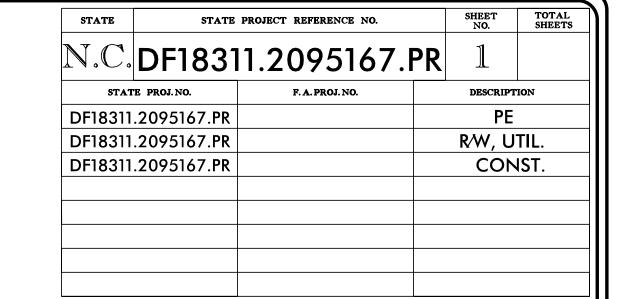
VICINITY MAP

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

WATAUGA COUNTY

LOCATION: BRIDGE #940058 OVER BEECH CREEK ON SR 1139 (KELLERSVILLE RD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE





-Y- STA. 11+00.00 BEGIN CONSTRUCTION	BEGIN BRIDGE -L- STA. 10 + 40.63 END BRIDGE -L- STA. 11 + 43.37	
DECII CONTROCII	_L_ STA. 12 + 50.00)
	END PROJECT	
	DF18311.2095167.F	P?
	730	
	Z \	
L STA. 10 + 11	.81 2	
BEGIN PROJEC	TILE RD	
DF18311.20951	67.PR	
-Y-ST	A. 13 + 00.00	
END (CONSTRUCTION	
	B	

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

GRAPHIC SCALES PLANS PROFILE (HORIZONTAL)

PROFILE (VERTICAL)

DESIGN DATA

ADT 2025 = 110ADT 2045 = 135

V = 30 MPH

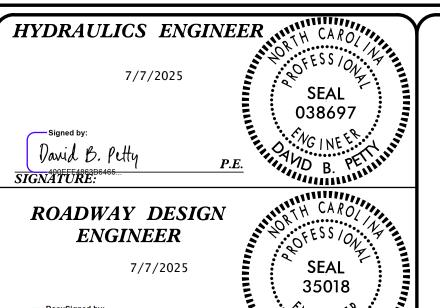
FUNC CLASS = LOCAL RURAL SUBREGIONAL TIER

PROJECT LENGTH

LENGTH OF ROADWAY PROJECT DF18311.2095167.PR = 0.026 MILES LENGTH OF STRUCTURE PROJECT DF18311.2095167.PR = 0.019 MILES

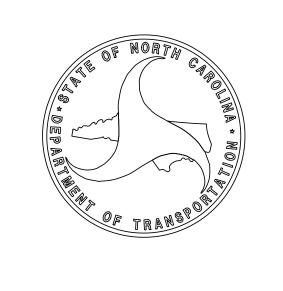
TOTAL LENGTH OF PROJECT DF18311.2095167.PR = 0.045 MILES NCDOT CONTACT: Rob Weisz, PE PLANS PREPARED BY: PLANS PREPARED FOR: NORTH CAROLINA DEPARTMENT TGS ENGINEERS OF TRANSPORATION 20 I W. MARION ST SHELBY, NC 28 150 DIVISION II 80 I Statesville Rd N. Wilkesboro, NC 28659 PH (704) 476-0003 CORP. LICENSE NO.: C-0275 RIGHT OF WAY DATE: JIMMY L. TERRY, PE MARCH 28, 2025 PROJECT ENGINEER LETTING DATE: SANDRA MELVIN AUGUST 21, 2025 PROJECT DESIGN ENGINEER

2024 STANDARD SPECIFICATIONS



Jimmy Terry

STGNATURE:400.



INDEX OF SHEETS

SHEET

CONVENTIONAL SYMBOLS

INDEX OF SHEETS, GENERAL NOTES, AND STANDARD DRAWINGS

PAVEMENT SCHEDULE AND TYPICAL SECTIONS

SPECIAL DETAILS - GUARDRAIL PLACEMENT

ROADWAY AND DRAINAGE SUMMARIES

GEOTECHNICAL SUMMARIES

SURVEY CONTROL SHEETS

TRAFFIC MANAGEMENT PLANS

PAVEMENT MARKING PLANS

EROSION CONTROL PLANS

CROSS-SECTION INDEX

REFORESTATION DETAIL SHEET

CROSS-SECTION SUMMARY SHEET

PLAN SHEET

PROFILE SHEET

SIGNING PLANS

CROSS-SECTIONS

STRUCTURE PLANS

SPECIAL DETAILS - METHOD OF PIPE INSTALLATION

SPECIAL DETAILS - TYPE III SHOP CURVED ANCHOR UNIT

SPECIAL DETAILS - B-83 SHOP CURVED ANCHOR UNIT

TITLE SHEET

SHEET NUMBER

2C-1 THRU 2C-2

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RW-01 THRU RW-04

TMP-1 THRU TMP-6

PMP-1 THRU PMP-2

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EC-1 THRU EC-5

RF-1

X-1

X-1A

X-2 THRU X-8

2A-1

2C-5

2C-6

3B-1

3G-1

PROJECT REFERENCE NO. SHEET NO. DF18311.2095167.PR

ROADWAY DESIGN **ENGINEER**

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EFF. 01-16-2024

2024 ROADWAY ENGLISH STANDARD DRAWINGS

The following Roadway Standards as appear in "Roadway Standard Drawings" Contracts Standards and Development Unit - N. C. Department of Transportation - Raleigh, N. C., Dated January 16, 2024 are applicable to this project and by reference hereby are considered a part of these plans:

TITLE STD.NO.

DIVISION 2 - EARTHWORK

200.03 Method of Clearing - Method III

225.02 Guide for Grading Subgrade - Secondary and Local

225.04 Method of Obtaining Superelevation - Two Lane Pavement

DIVISION 3 - PIPE CULVERTS

300.01 Method of Pipe Installation

(Use Details in Lieu of Standards for Sheets 1 and 2 of 2)

310.10 Driveway Pipe Construction

DIVISION 4 - MAJOR STRUCTURES

423.01 Bridge Approach Fills - Type 1 Approach Fill for Bridge Abutment

DIVISION 5 - SUBGRADE, BASES AND SHOULDERS

560.01 Method of Shoulder Construction

- High Side of Superelevated Curve - Method I

DIVISION 8 - INCIDENTALS

Subsurface Drain

Concrete Base Pad for Drainage Structures

840.29 Frames and Narrow Slot Flat Grates

840.35 Traffic Bearing Grated Drop Inlet

- for Cast Iron Double Frame and Grates

840.46 Traffic Bearing Precast Drainage Structure

846.01 Concrete Curb, Gutter and Curb & Gutter 846.04 Drop Inlet Installation in Shoulder Berm Gutter

862.01 Guardrail Placement

(Use Details in Lieu of Standards for Sheets 4, 6, 12, and 14 of 15)

862.02 Guardrail Installation 862.03 Structure Anchor Units

(Use Detail in Lieu of Standard for Sheet 8 of 9)

862.04 Anchorina End of Guardrail - for B-77 and B-83 Anchor Units

GENERAL NOTES:

2024 SPECIFICATIONS EFFECTIVE: 01-16-2024 REVISED:

GRADING AND SURFACING OR RESURFACING AND WIDENING:

THE GRADE LINES SHOWN DENOTE THE FINISHED ELEVATION OF THE PROPOSED SURFACING AT GRADE POINTS SHOWN ON THE TYPICAL SECTIONS. WHERE NO GRADE LINES ARE SHOWN, THE PROFILES SHOWN DENOTE THE TOP ELEVATION OF THE EXISTING PAVEMENT ALONG THE CENTER LINE OF SURVEY ON WHICH THE PROPOSED RESURFACING WILL BE PLACED. GRADE LINES MAY BE ADJUSTED BY THE ENGINEER IN ORDER TO SECURE A PROPER TIE-IN.

CLEARING:

CLEARING ON THIS PROJECT SHALL BE PERFORMED TO THE LIMITS ESTABLISHED BY METHOD III.

SUPERELEVATION:

ALL CURVES ON THIS PROJECT SHALL BE SUPERELEVATED IN ACCORDANCE WITH STD. NO. 225.04 USING THE RATE OF SUPERELEVATION AND RUNOFF SHOWN ON THE PLANS. SUPERELEVATION IS TO BE REVOLVED ABOUT THE GRADE POINTS SHOWN ON THE TYPICAL SECTIONS.

SHOULDER CONSTRUCTION:

ASPHALT, EARTH, AND CONCRETE SHOULDER CONSTRUCTION ON THE HIGH SIDE OF SUPERELEVATED CURVES SHALL BE IN ACCORDANCE WITH STD. NO. 560.01

SIDE ROADS:

THE CONTRACTOR WILL BE REQUIRED TO DO ALL NECESSARY WORK TO PROVIDE SUITABLE CONNECTIONS WITH ALL ROADS, STREETS, AND DRIVES ENTERING THIS PROJECT. THIS WORK WILL BE PAID FOR AT THE CONTRACT UNIT PRICE FOR THE PARTICULAR ITEMS INVOLVED.

SUBSURFACE DRAINS:

SUBSURFACE DRAINS SHALL BE CONSTRUCTED IN ACCORDANCE WITH STD, NO. 815,02 AT LOCATIONS DIRECTED BY THE ENGINEER.

GUARDRAIL:

THE GUARDRAIL LOCATIONS SHOWN ON THE PLANS MAY BE ADJUSTED DURING CONSTRUCTION AS DIRECTED BY THE ENGINEER. THE CONTRACTOR SHOULD CONSULT WITH THE ENGINEER PRIOR TO ORDERING GUARDRAIL MATERIAL.

TEMPORARY SHORING:

SHORING REQUIRED FOR THE MAINTENANCE OF TRAFFIC NOT SHOWN ON THE PLANS WILL BE PAID FOR AT THE CONTRACT PRICE FOR "TEMPORARY SHORING".

END BENTS:

THE ENGINEER SHALL CHECK THE STRUCTURE END BENT PLANS, DETAILS, AND CROSS-SECTION PRIOR TO SETTING OF THE SLOPE STAKES FOR THE EMBANKMENT OR EXCAVATION APPROACHING A BRIDGE.

UTILITIES:

UTILITY OWNERS ON THIS PROJECT ARE SKYLINE

ANY RELOCATION OF EXISTING UTILITIES WILL BE ACCOMPLISHED BY OTHERS.

RIGHT-OF-WAY MARKERS:

ALL RIGHT-OF-WAY MARKERS ON THIS PROJECT SHALL BE PLACED BY OTHERS.

S-1 THRU S-26

STANDARD STRUCTURE NOTES

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STATE OF NORTH CAROLINA, DIVISION OF HIGHWAYS

DF18311,2095167,PR

05/20/24

Note: Not to Scale

State Line ————————————————————————————————————	
County Line ————————————————————————————————————	
Township Line	
City Line ————————————————————————————————————	
Reservation Line	
Property Line ————————————————————————————————————	
Existing Iron Pin (EIP)	EIP
Computed Property Corner —————	×
Existing Concrete Monument (ECM)	ECM
Parcel / Sequence Number ————	
Existing Fence Line	×××
Proposed Woven Wire Fence ————	
Proposed Chain Link Fence	
Proposed Barbed Wire Fence	
Existing Wetland Boundary —————	
Proposed Wetland Boundary ————	
Existing Endangered Animal Boundary—	
Existing Endangered Plant Boundary —	
Existing Historic Property Boundary ——	
Known Contamination Area: Soil ———	
Potential Contamination Area: Soil ———	
Known Contamination Area: Water ——	
Potential Contamination Area: Water —	
Contaminated Site: Known or Potential -	
BUILDINGS AND OTHER CULTU	
Gas Pump Vent or U/G Tank Cap	
Sign ————————————————————————————————————	_
Small Mine	
Foundation —	
Area Outline ————————————————————————————————————	
Cemetery ————————————————————————————————————	
Building	
School	
Church ————————————————————————————————————	— <u>4</u>
Dam —	
HYDROLOGY:	
Stream or Body of Water —————	
Hydro, Pool or Reservoir———————————————————————————————————	
Jurisdictional Stream	JS
Buffer Zone 1 ———————————————————————————————————	BZ 1
Buffer Zone 2 ———————————————————————————————————	——— BZ 2 ———
Flow Arrow —————	
TIOW AITOW	
Disappearing Stream —	<u> </u>
Disappearing Stream —————	,
Disappearing Stream ————————————————————————————————————	

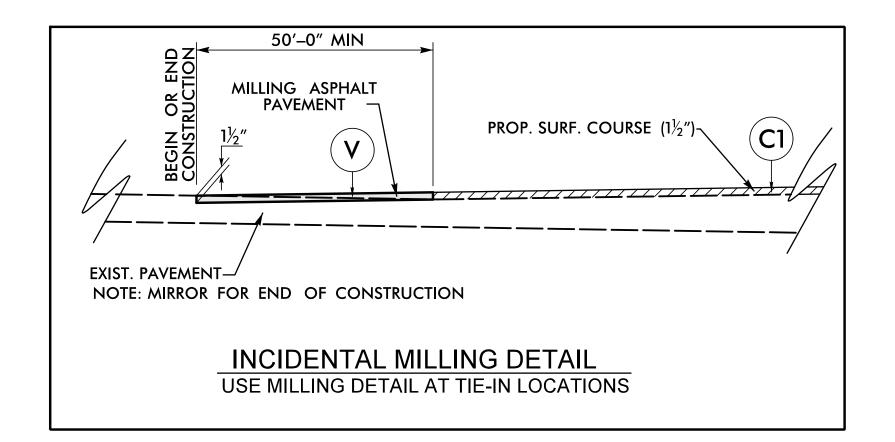
CONVENTIONAL PLAN SHEET SYMBOLS

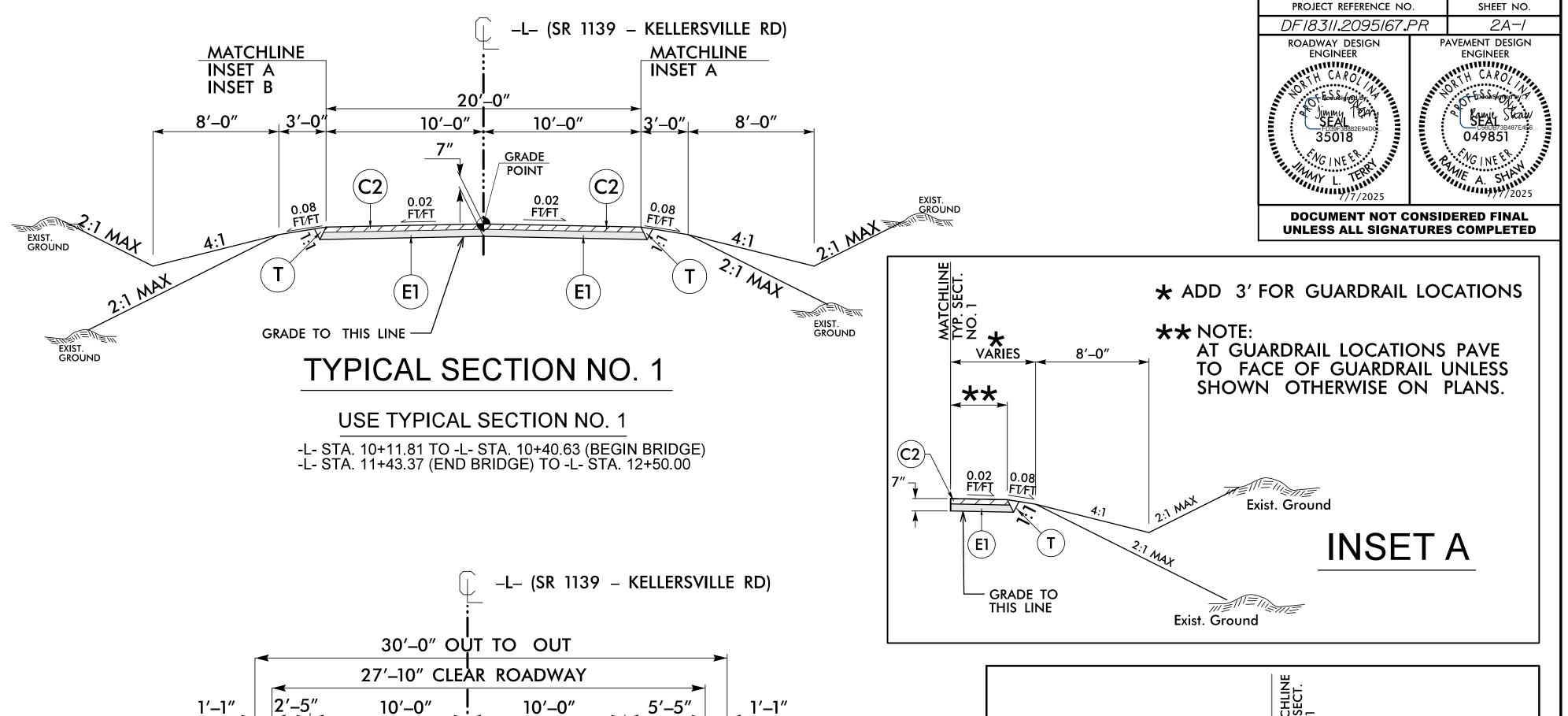
RAILROADS:	
Standard Gauge ————	CSX TRANSPORTATION
RR Signal Milepost —————	⊙ MILEPOST 35
Switch —	SWITCH
RR Abandoned ————	
RR Dismantled ————	
RIGHT OF WAY & PROJECT CONTE	ROL:
Primary Horiz and Vert Control Point ———	•
Secondary Horiz and Vert Control Point ——	•
Vertical Benchmark ————	
Existing Right of Way Monument———	\triangle
Proposed Right of Way Monument————————————————————————————————————	
Proposed Right of Way Monument (Concrete)	
Existing Permanent Easement Monument —	\bigcirc
Proposed Permanent Easement Monument — (Rebar and Cap)	•
Existing C/A Monument —————	\triangle
Proposed C/A Monument (Rebar and Cap)—	A
Proposed C/A Monument (Concrete) ———	
Existing Right of Way Line	
Proposed Right of Way Line ————	
Existing Control of Access Line ————	
Proposed Control of Access Line ————————————————————————————————————	
Proposed ROW and CA Line ————————————————————————————————————	
Proposed Temporary Construction Easement-	
Proposed Temporary Construction Lasement —	
Proposed Permanent Drainage Easement —	
Proposed Permanent Drainage Lasement — Proposed Permanent Drainage/Utility Easement	
Proposed Permanent Utility Easement ———	
Proposed Termanent Othity Easement ————————————————————————————————————	
Proposed Aerial Utility Easement———	
·	——AUE——
ROADS AND RELATED FEATURES:	
Existing Edge of Pavement ————	
Existing Curb ————————————————————————————————————	
Proposed Slope Stakes Cut ————	
Proposed Slope Stakes Fill ————	
Proposed Curb Ramp	
Existing Metal Guardrail ————	
Proposed Guardrail ————	
Existing Cable Guiderail	
Proposed Cable Guiderail ————	_
Equality Symbol ————	•
Pavement Removal	XXXXXX
VEGETATION:	^
Single Tree —————————————————————————————————	& -
Single Shrub ————————————————————————————————————	\$
Hedge ————	······································

Woods Line —	
Orchard —	—
Vineyard	— Vineyard
EXISTING STRUCTURES:	
MAJOR:	
Bridge, Tunnel or Box Culvert ————————————————————————————————————	- CONC
Bridge Wing Wall, Head Wall and End Wall	-) conc ww [
MINOR:	
Head and End Wall	
Pipe Culvert	
Footbridge — — — — — — — — — — — — — — — — — — —	
Drainage Box: Catch Basin, DI or JB	
Paved Ditch Gutter	
Storm Sewer Manhole	
Storm Sewer	s
UTILITIES:	
* SUE - Subsurface Utility Engineering LOS - Level of Service - A,B,C or D (
POWER:	Accuracy)
Existing Power Pole	_ •
Proposed Power Pole ————————————————————————————————————	•
Existing Joint Use Pole	1
Proposed Joint Use Pole —————	•
Power Manhole	
Power Line Tower ————————————————————————————————————	
Power Transformer ———————————————————————————————————	
U/G Power Cable Hand Hole	
H-Frame Pole	
U/G Power Line Test Hole (SUE - LOS A)* -	_
U/G Power Line (SUE - LOS B)*	
U/G Power Line (SUE - LOS C)*	
U/G Power Line (SUE - LOS D)*	
TELEPHONE:	
Existing Telephone Pole	- - ←
Proposed Telephone Pole ————————————————————————————————————	
Telephone Manhole	
Telephone Pedestal	
Telephone Cell Tower ————————————————————————————————————	_
U/G Telephone Cable Hand Hole ———	
U/G Telephone Test Hole (SUE - LOS A)* -	
U/G Telephone Cable (SUE - LOS B)* ——	t
U/G Telephone Cable (SUE - LOS C)*	
U/G Telephone Cable (SUE - LOS D)*	т
U/G Telephone Conduit (SUE - LOS B)* —	— — — тс— — —
U/G Telephone Conduit (SUE - LOS C)*	тс
U/G Telephone Conduit (SUE - LOS D)*	тс
U/G Fiber Optics Cable (SUE - LOS B)*	— — — — T FO— — — ·
U/G Fiber Optics Cable (SUE - LOS C)*	
•	

WATER:	
Water Manhole —	W
Water Meter————	0
Water Valve —	\otimes
Water Hydrant ————	⊹
U/G Water Line Test Hole (SUE - LOS A)* —	S
U/G Water Line (SUE - LOS B)*	w
U/G Water Line (SUE - LOS C)*	
U/G Water Line (SUE - LOS D)*	
Above Ground Water Line —	
TV:	
TV Pedestal	C
TV Tower —	\otimes
U/G TV Cable Hand Hole ————	H _H
U/G TV Test Hole (SUE - LOS A)*	
,	Tv
U/G TV Cable (SUE - LOS C)*	тv
U/G TV Cable (SUE - LOS D)*	TV
U/G Fiber Optic Cable (SUE - LOS B)* ——	
U/G Fiber Optic Cable (SUE - LOS C)* ——	
U/G Fiber Optic Cable (SUE - LOS D)* ——	
GAS:	
Gas Valve ————	\Diamond
Gas Meter ————	\Diamond
U/G Gas Line Test Hole (SUE - LOS A)* —	•
U/G Gas Line (SUE - LOS B)*	
U/G Gas Line (SUE - LOS C)*	
U/G Gas Line (SUE - LOS D)*	
Above Ground Gas Line ————	
SANITARY SEWER:	
Sanitary Sewer Manhole ————	(h)
Sanitary Sewer Cleanout —————	(+)
U/G Sanitary Sewer Line	ss
Above Ground Sanitary Sewer	A/G Sanitary Sewer
SS Force Main Line Test Hole (SUE - LOS A)*	
SS Force Main Line (SUE - LOS B)* ———	— — — FSS— — — –
SS Force Main Line (SUE - LOS C)*	——————————————————————————————————————
SS Force Main Line (SUE - LOS D)*	FSS
MISCELLANEOUS:	
Utility Pole —————	•
Utility Pole with Base	
Utility Located Object —————	\odot
Utility Traffic Signal Box ————	S
Utility Unknown U/G Line (SUE - LOS B)* —	
U/G Tank; Water, Gas, Oil ————	
Underground Storage Tank, Approx. Loc.——	(UST)
A/G Tank; Water, Gas, Oil —————	
Geoenvironmental Boring —————	─
Abandoned According to Utility Records ——	AATUR
End of Information	E.O.I.

PAVEMENT EDGE SLOPES ARE 1:1 UNLESS OTHERWISE SHOWN.





ASPHALT OVERLAY

1-BAR METAL RAIL

VARIES

USE INSET B

-Y- STA. 11+59.90 LT TO -L- STA. 10+21.67 LT (BEGIN APPROACH SLAB)

INSET B

- GRADE TO THIS LINE

TYPICAL SECTION NO. 2

10 - 24" CORED SLAB UNITS = 30'-0"

GRADE

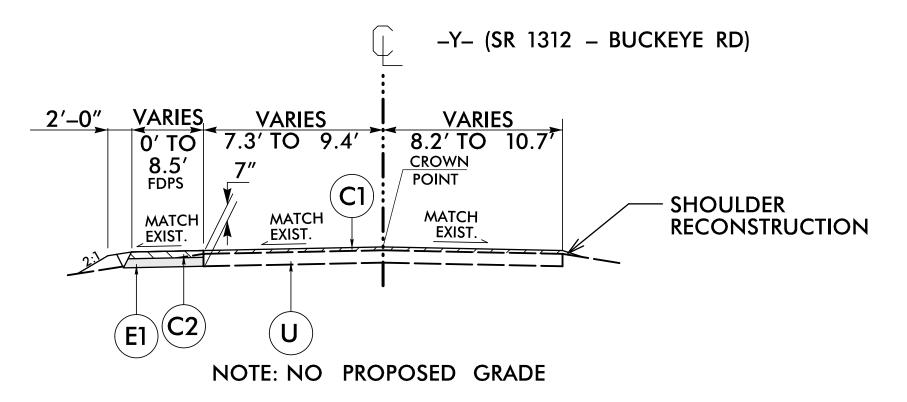
FT/FT

 $1|\frac{1}{2}$ " MIN.

FT/FT

1-BAR METAL RAIL

USE TYPICAL SECTION NO. 2
-L- STA. 10+40.63 TO -L- STA. 11+43.37



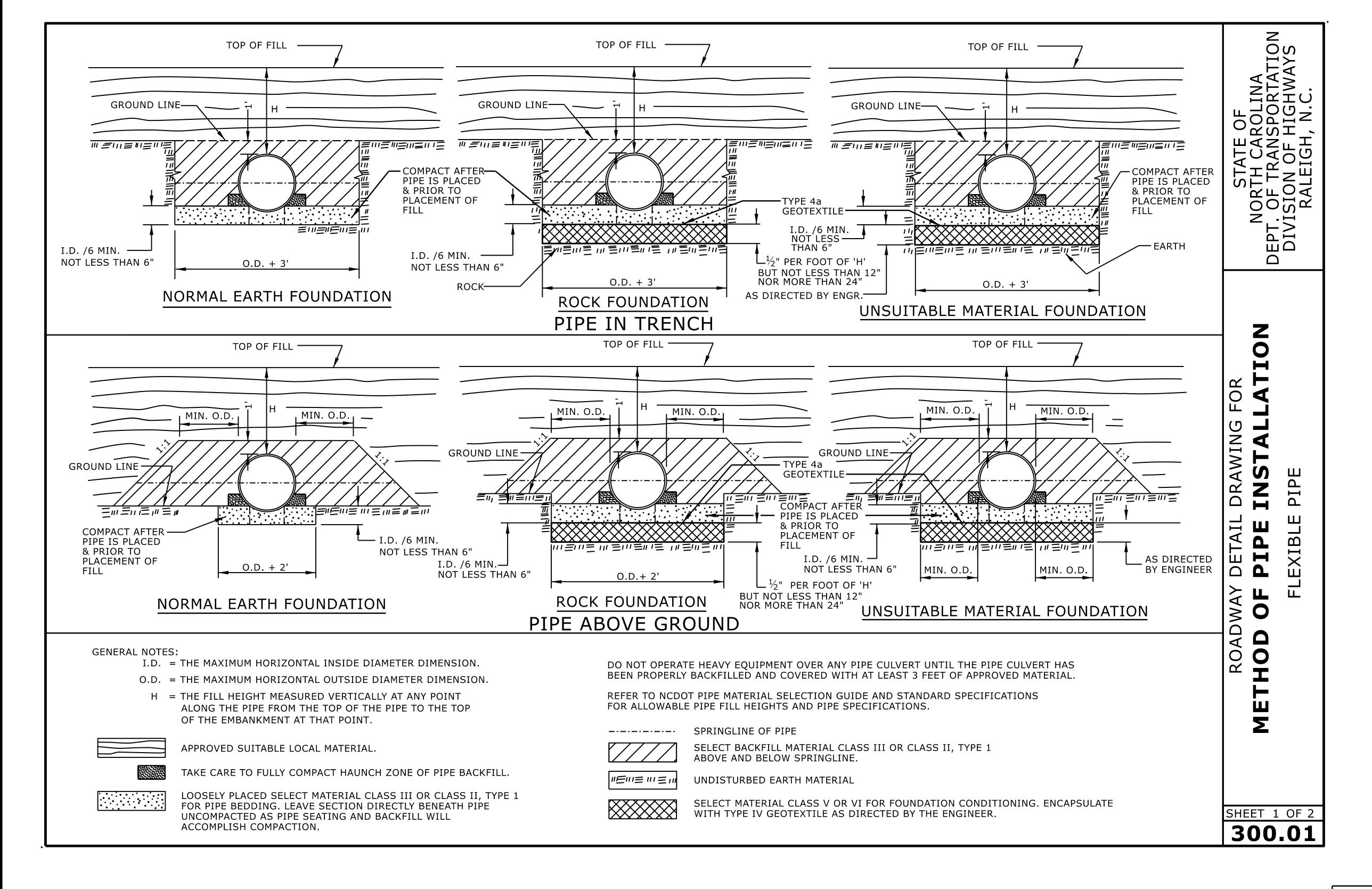
TYPICAL SECTION NO. 3

USE TYPICAL SECTION NO. 3
-Y- STA. 11+00.00 TO -Y- STA. 13+00.00

"SMe_V1D

PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR 2C-1



CARO

Signadori

Nicole A Hackler

SEGAN-0034164C5

ONGINE

7/7/2021

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CONTRACTS STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

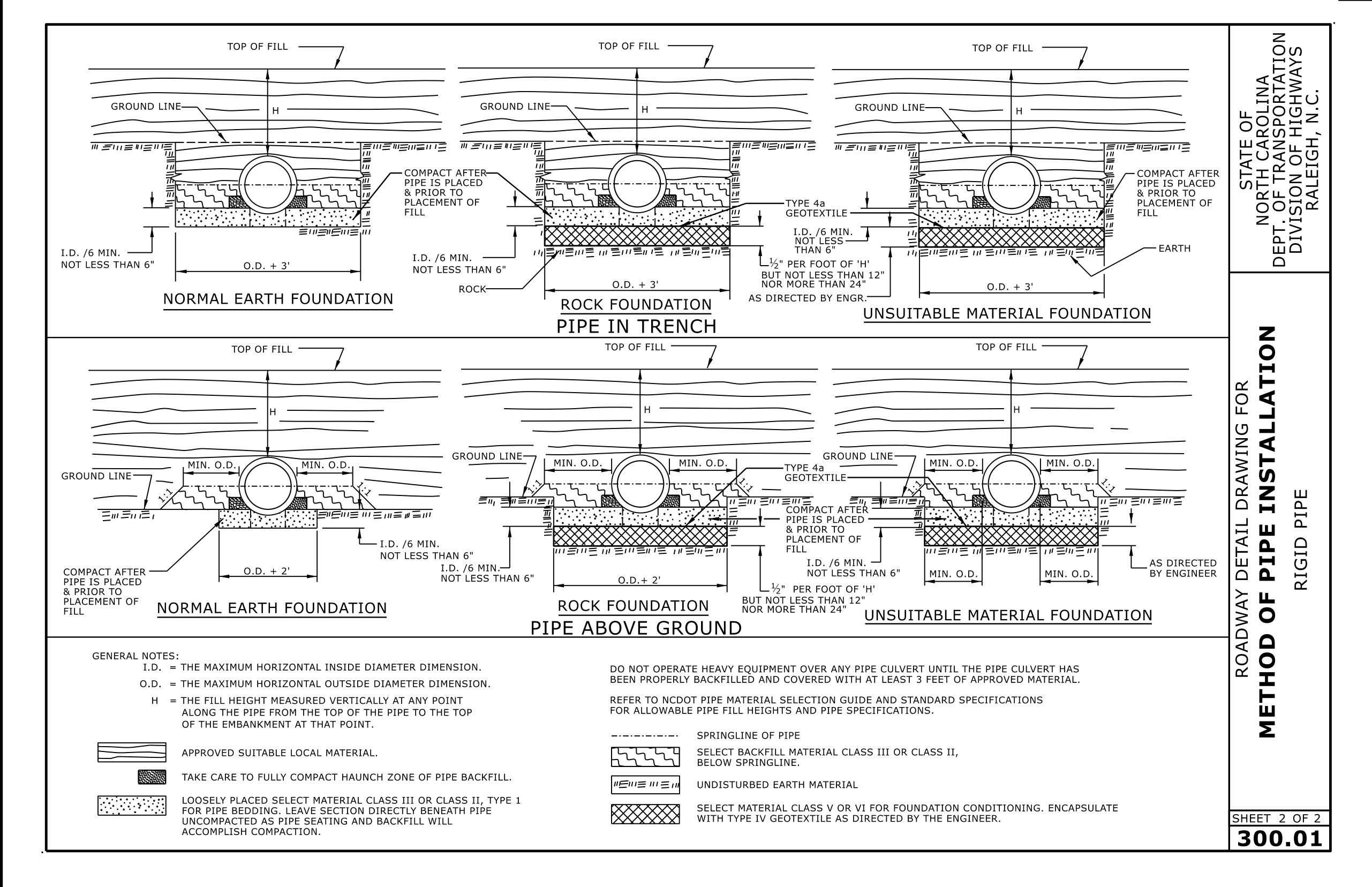
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MODIFIED BY: DATE: DATE: FILE SPEC.:

PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR 2C-2



CARO
Signed by
Nicole M. Hacker
SEA 3534164C5...

Z 033144

7/7/2025

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MODIFIED BY: DATE: DATE: FILE SPEC.:

PROJECT REFERENCE NO DF18311.2095167.PR 2C-3 DETAIL

28' MIN. STRUCTURE ANCHOR UNIT 50:1 OR FLATTER FLARE RATE GUARDRAIL END UNIT TAPER TYPE TL-3 or TL-2 PARALLEL TO (50:1 TAPER) 2'-LANE SHOULDER LINE — EDGE OF LANE -EDGE OF LANE SHOULDER LINE —— STRUCTURE ANCHOR UNIT 50:1 OR FLATTER **GUARDRAIL END UNIT** TYPE TL-3 or TL-2 (50:1 TAPER) PARALLEL TO FLARE RATE TAPER LANE 28' MIN. USE FLARE RATE AS THE CONTROL IF THE "N₁" DISTANCE IS NOT OBTAINED. ("N₁" IS BASED ON SHOULDER WIDTHS IN THE ROADWAY DESIGN MANUAL) SEE STD. 862.03 FOR STRUCTURE ANCHOR UNITS FOR POSTED SPEEDS ≥ 45MPH USE GREU TYPE TL-3 FOR POSTED SPEEDS < 45MPH USE GREU TYPE TL-2 GUARDRAIL LENGTH OF NEED (X) IS CALCULATED BASED ON THE AASHTO ROADSIDE DESIGN GUIDE.

LENGTHS AND OFFSETS FOR PROPOSED GUARDRAIL AT TWO LANE - TWO WAY LOCATIONS

SHEET 4 OF 15 862D01

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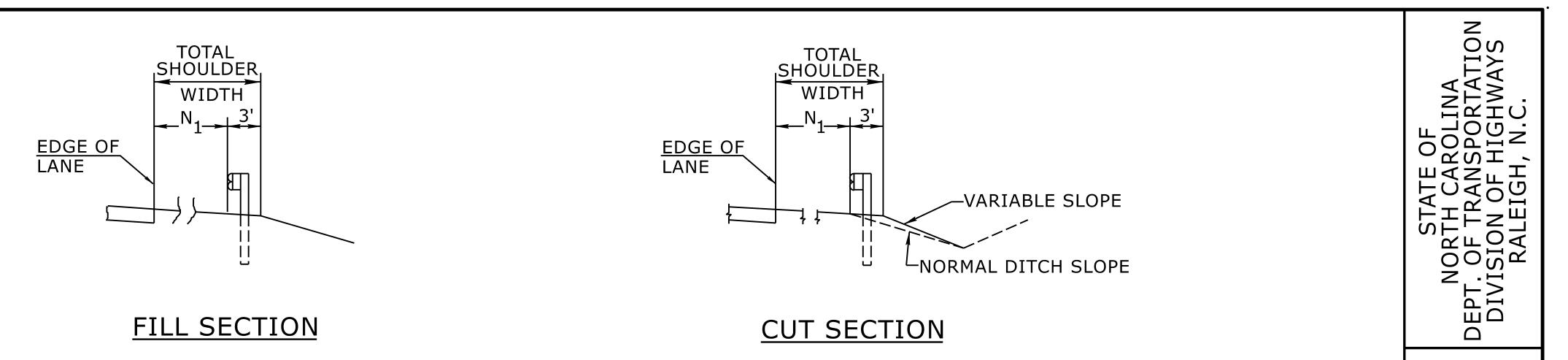
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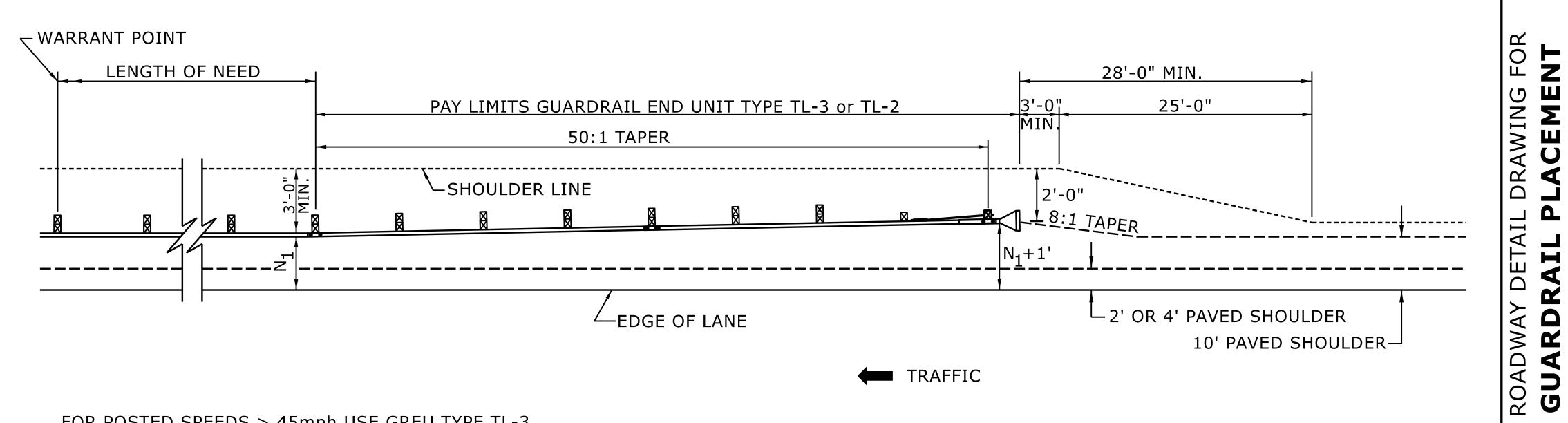
ORIGINAL BY: S.CALHOUN DATE: 7-25-2024

MODIFIED BY: DATE: DATE: FILE SPEC.:

DF18311.2095167.PR



"N₁"= DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL WHERE GUARDRAIL IS PARALLEL TO LANE.



FOR POSTED SPEEDS ≥ 45mph USE GREU TYPE TL-3 FOR POSTED SPEEDS < 45mph USE GREU TYPE TL-2

DETAIL OF BEGINNING OF GUARDRAIL IN CUT OR FILL SECTION



SHEET 6 OF 15

862D01

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MODIFIED BY:		_ DATE:	
CHECKED BY:		_ DATE:	

 PROJECT REFERENCE NO.
 SHEET NO.

 DF18311.2095167.PR
 2C-5

STA NORTH . OF T /ISION RALEJ PAY LIMITS 3'-11/2" SEE PLANS STD. 6'-3" SPACING THRIE BEAM GUARDRAIL 'NESTED' WTR SECTION MIDSPAN SPLICE -----STA NORTH OF T 'ISION RALEI SPORTATION HIGHWAYS N.C. FINISH GRADE FINISH CONCRETE BACKWALL SEE ROADWAY PLANS FOR END TREATMENT GRADE FILL FACE \setminus 4 $^{\prime\prime}$ x 8 $^{\prime\prime}$ APPROACH SLAB LIP CURB APPROACH SLAB **ELEVATION** NOTE: **POST NOT REQUIRED FOR SKEW ANGLES GREATER THAN 150° OR LESS THAN 30° UNLESS OTHERWISE DIRECTED BY THE ENGINEER. *THE DISTANCE FROM END OF BRIDGE RAIL TO CENTER LINE OF THE FIRST POST SHOULD BE $11\frac{1}{2}$ " IF CONCRETE BACKWALL IS NOT PRESENT. CURVED R UNIT ENGL -SHOULDER BERM GUTTER MUST BE INSTALLED TO THE LIMITS 8" x 4" LIP CURB IS SHOWN IF ANCHOR UNIT IS NOT ADJACENT FOR TRUC' TO AN APPROACH SLAB -MEASURE GUARDRAIL HEIGHT FROM THE TOP OF ADJACENT SURFACE (SHOULDER, BERM, OR GUTTER). -USE NO STEEL POSTS WITHIN THE GUARDRAIL ANCHOR UNIT LIMITS. DRAWING -LAP JOINTS IN THE DIRECTION OF TRAFFIC FLOW. TURE -SEE STANDARD 862.03 SHEET 4 FOR POST SECTIONS 1 THRU 9. ANCHO SHOP ADDITIONAL PAVED SHOULDER VERTICAL PLANE AT THE ATTACHMENT POINT FOR END SHOE ANCHORAGE, ETAIL SEE STRUCTURE PLANS HO TYPE III STRUCTUR AWING OR BRIDGE RAIL **ENGLISH** URVED SHOP CURVED GUARDRAIL
SEE ROADWAY PLANS OR AS
DIRECTED BY ENGINEER FOR APPROACH SLAB **PLAN VIEW** GUARDRAIL ANCHOR UNIT, TYPE III - SHOP CURVED FOR ATTACHMENT TO RAIL ON BRIDGE SHEET 1 OF 1 SHEET 1 OF 1 TYPE III SC TYPE III SC



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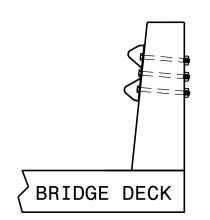
CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

SEE PLATE FOR TITLE

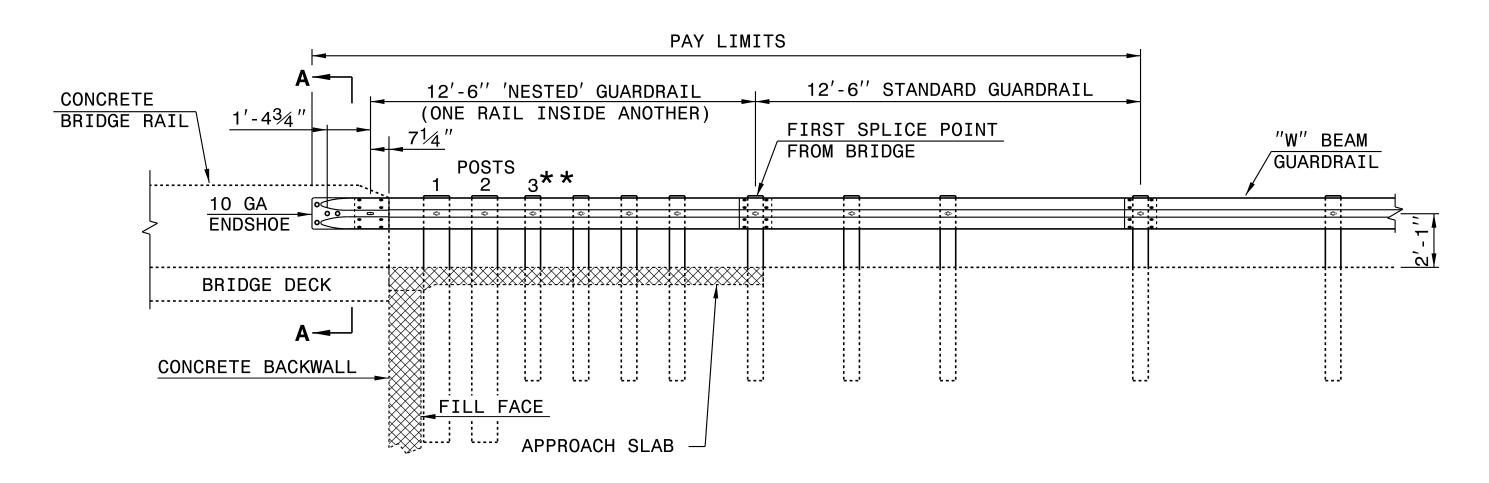
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PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR 2C-6



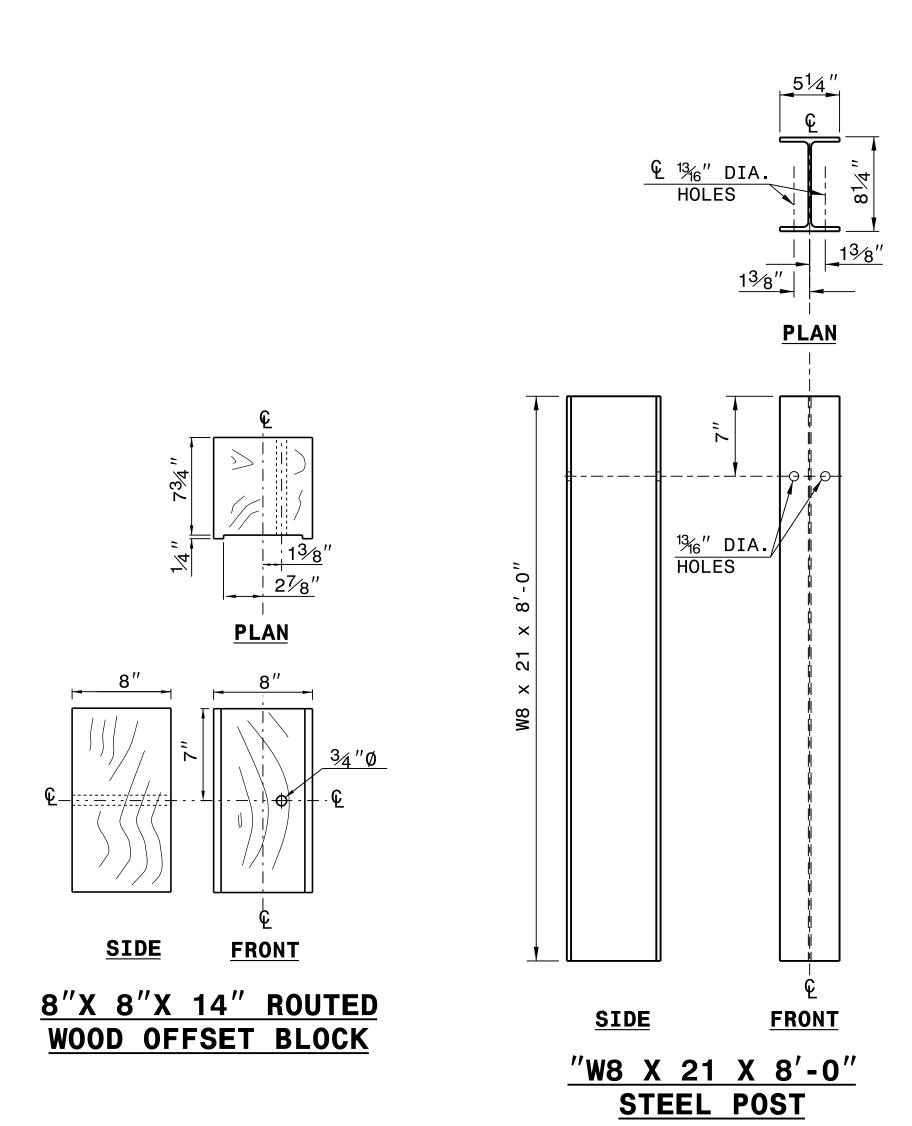
SECTION A-A

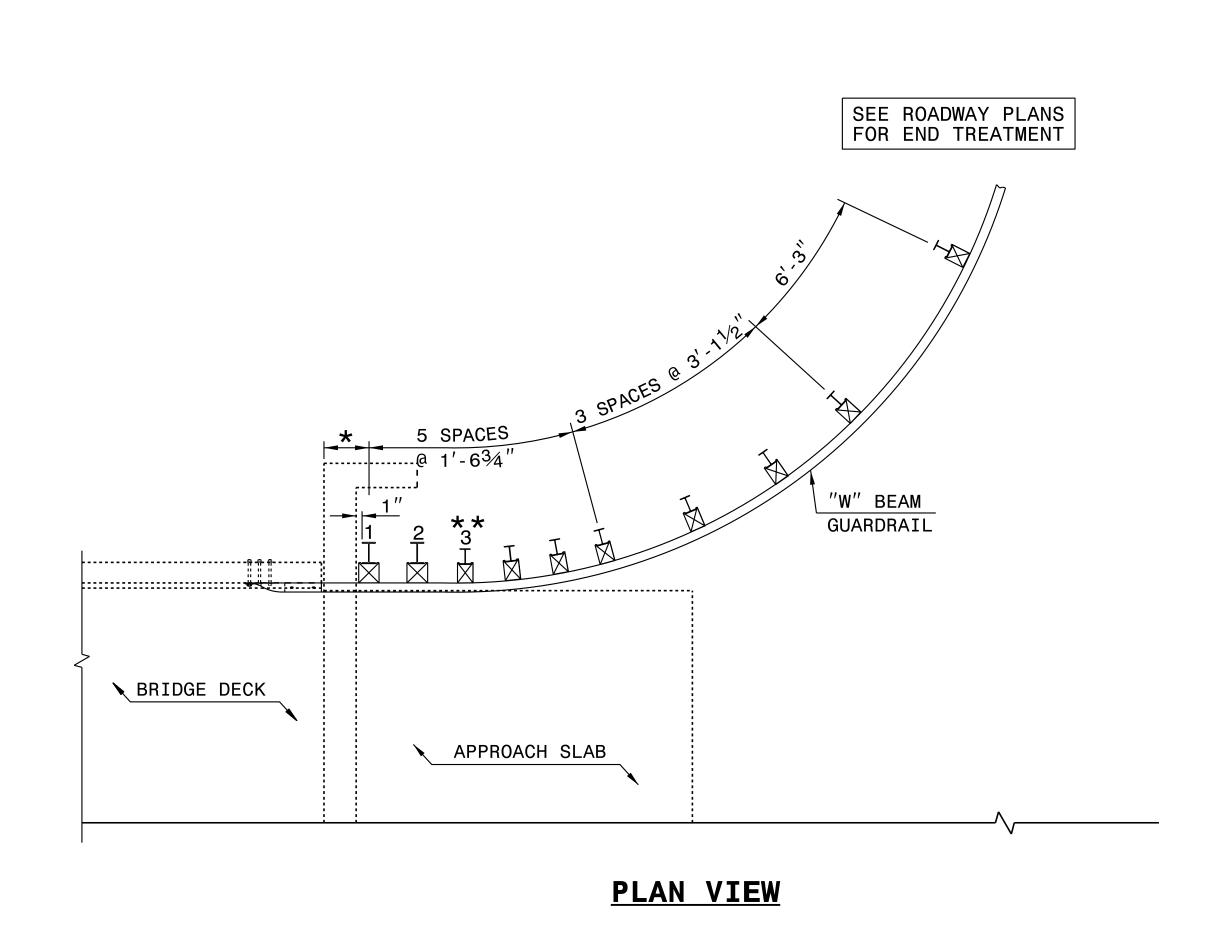


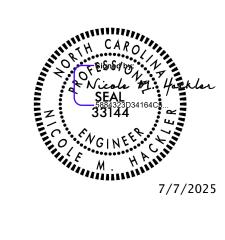
ELEVATION VIEW

NOTE:

- **ELIMINATE POST 3 AND SHIFT POSTS 1 & 2 ON SKEW ANGLES GREATER THAN 150° OR LESS THAN 30° UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- *THE DISTANCE FROM END OF BRIDGE RAIL TO CENTER LINE OF THE FIRST POST SHOULD BE $11\frac{1}{2}$ " IF CONCRETE BACKWALL IS NOT PRESENT.
- -MEASURE GUARDRAIL HEIGHT FROM THE TOP OF ADJACENT SURFACE (SHOULDER, BERM, OR GUTTER).
- -USE NO WOOD POSTS WITHIN THE GUARDRAIL ANCHOR UNIT LIMITS.
- -LAP JOINTS IN THE DIRECTION OF TRAFFIC FLOW.
- -POSTS 1 AND 2 TO BE W8 x 21 X 8'-0" LONG STEEL POST AND 8" X 8" X 14" WOOD ROUTED OFFSET BLOCK.







DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

CONTRACT STANDARDS AND DEVELOPMENT UNIT Office 919-707-6950 FAX 919-250-4119

GUARDRAIL ANCHOR UNIT TYPE B-83 SHOP CURVED

ORIGINAL BY: E.E. WARD DATE: 6-10-02

MODIFIED BY: E.E. WARD DATE: 7-14-04

CHECKED BY: DATE: FILE SPEC.:

CSD-292595

COMPUTED BY: SGM DATE: 5/30/2025 CHECKED BY: JLT DATE: 7/7/2025

STATE OF NORTH CAROLINA

PROJECT NO. SHEET NO. DF18311.2095167.PR 3B-1

SUMMARY OF EARTHWORK

IN CUBIC YARDS

	IN COBIC	, TARDS			
Station	Station	Uncl. Excav.	Embank. +%	Borrow	Waste
-Y- 11+00.00	-Y- 13+00.00	0	37	37	
-L- 10+11.81	-L- 10+40.63	3	62	59	
BRIDGE					
SUBT	OTAL:	3	99	96	
-L- 11+43.37	-L- 12+50.00	14	107	93	
SUBT	OTAL:	14	107	93	
тот	ALS:	17	206	189	
LOSS DUE TO CLEARING & G	GRUBBING				
PROJECT	TOTALS:	17	206	189	0
EST. 5% TO REPLACE TOP SO	OIL ON BORROW PIT			9	
GRAND	TOTALS:	17	206	198	0
SA	AY	50		250	

Note: Approximate quantities only. Unclassified Excavation, Borrow Excavation, Fine Grading, Clearing and Grubbing, and Removal of Existing Pavement will be paid for at the contract lump sum price for grading.

Note: Earthwork quantities are calculated by TGS Engineers. These earthwork quantities are based in part on subsurface data provided by the Geotechnical Engineering Unit.

SELECT GRANULAR MATERIAL = 400 CUBIC YARDS SHALLOW UNDERCUT = 100 CUBIC YARDS PER GEOTECH RECOMMENDATION, ESTIMATED 400 CUBIC YARDS OF UNDERCUT TO BE USED IN THE DISCRETION OF THE RESIDENT ENGINEER.

SHOULDER BERM GUTTER SUMMARY

IN LINEAR FEET

LINE	STATION	STATION	LENGTH
-L-, LT	11+59.90 -Y-	10+21.67	34.70
		TOTAL:	34.70
		SAY	35

PAVEMENT REMOVAL SUMMARY

IN SQUARE YARDS

LINE	STATION	STATION	LT/RT/CL	ASPHALT REMOVAL	ASPHALT BREAKUP	CONCRETE REMOVAL	CONCRETE BREAKUP
-L-	11+35	12+50	CL	253.07			
-L-	Asphalt Pad at	Гетр. Bridge		66.46			
-L-	Asphalt Pad at	Гетр. Bridge		29.35			
			TOTAL:	348.88			
			SAY	350			

"N" = DISTANCE FROM EDGE OF LANE TO FACE OF GUARDRAIL
TOTAL SHOULDER WIDTH = DISTANCE FROM EDGE OF TRAVEL LANE TO SHOULDER BREAK POINT.
LENGTH = DISTANCE FROM LAST SECTION OF PARALLEL GUARDRAIL TO END OF GUARDRAIL. W = TOTAL WIDTH OF FLARE FROM BEGINNING OF TAPER TO END OF GUARDRAIL. G = GATING IMPACT ATTENUATOR TYPE 350 NG = NON-GATING IMPACT ATTENUATOR TYPE 350

FLARE

GUARDRAIL SUMMARY

					LENGTH	WARRA	NT POINT	"N" DIST.	TOTAL		ENGTH	W				ANCHORS		IMP. ATTEN.		REMOVE EXISTIN	c
LINE	BEG. STA.	END STA.	LOC.	STRAIGHT	SHOP CURVED DOUBLE FACED	APPR. END	TRAIL. END	FROM E.O.L.	SHLDR WIDTH	APPR. END	TRAIL. END	APPR. END	TRAIL. END	TYPE III SC	TL-3	AT-1	B-83 SC	350 EA G	Extra Depth I	osts GUARDRAIL	REMARKS
-Y-/-L-	11+10	10+34.76	LT	50.00	43.75		10+34.76	2.42'	5.42'		50'		1'	1	1						R=47'
-L-/-Y-	10+50.83	12+95.00	RT/LT	56.25	25	10+50.83		5.42'	8.42'								1		9		R=10', Tie to exist. Guardrail; Extra Depth post: 56.25 ft 6'-3" = 9 ea,
-L-	11+52.90	11+90.25	RT		31.25		11+52.90	5.42'	8.42'					1		1					R=25'
-L-	11+34.90	11+47.00	LT		31.25	11+34.90		2.42'	5.42'					1		1					R=15'
-Y-	12+52.00	12+95.00	LT																	45	
		SUB-TOTALS		106.25	131.25									3	1	2	1		9	45	
	LESS ANCHOR DEDUCTIONS																				
	TYPE III Shop Curved	3 @ 18.75 ft			56.25																
	TYPE TL-3	1 @ 50.00 ft		50.00																	
	AT-1	2 @ 6.25 ft			12.5																
	B-83 Shop Curved	1 @ 25.00 ft			25																
	ANCHOR TOTALS			50.00	93.75																
		GRAND-TOTALS		56.25	37.50									3	1	2	1		9	45	
				62.50	50									3	1	2	1		10	45	
			ADDITIONAL O	GUARDRAIL P	POSTS = 5 EA								<u></u>								

Note: Invert Elevations indicated are for Bid Purposes only and shall not be used for project construction stakeout. See "Standard Specifications For Roads and Structures, Section 300-5".

LICT OF DIDEC FURILILIS FTC (FOR DIDEC 10 INCHES 0 LINDED)

COMPUTED BY: Kathle	en Gray, PE	DATE: 6/5/2025	
CHECKED BY: Zachary			

												LIST	T OF P	PIPES,	, END	WALL	S, ETC.	(FOR	R PIPE	S 48 L	NCH	ES &	UND	ER)														
STATION		TURE NO.		ATION	VATION	VATION	TICAL	(F	DRAINAGE RCP, CSP, CAAP,	E PIPE HDPE, or PVC)			C.S. PIPE				C. PIPE ASS III		F C	R.C. PIPE CLASS IV		TOR DESIGN TOR DESIGN	D. 838.01 DR 8.80	OTHERWISE) NDWALLS	STRUC *TOTAL L.I QUANTITY SI	NTITIES RAINAGE CTURES F. FOR PAY HALL BE COL. X COL.'B')		FRAME, GRAT		SECTION	J. 840.27	ES STD. 840.29			D. 840.71	840.72	C.B. N.D.I. D.I. G.D.I.	CATCH BASIN NARROW DROP INLET DROP INLET GRATED DROP INLET
SIZE PO		STRUC		TOP ELEW	INVERT ELEY	INVERT ELEY	SLOPE CRI	12" 15" 18" 24	." 30" 36"	42" 48" "24 401 USE RCP	OT USE CAAP	12" 15"	18" 24" 30"	36" 42"	48" 12" 15'	18" 24"	30" 36" 42"	' 48" 12"	15" 18" 24'	" 30" 36"		: (CLASS V) CULVERTS, CONTRAC CULVERTS, CONTRAC IN PIPE	ENDWALLS ST ENDWALLS ST 838.11 (STD. 838	(UNLESS NOTED (ACH (0' THRU	IRU 10.0' Y	.01 OR STD. 840.02	STANDARD 84	0.03	14 OR STD. 840.15	B" STD. 840.18 OR STE 840.35	LAT)FRAME W/2 GRAT	840.35		CK PIPE PLUG, C.Y. ST	"B" C.Y. ST		(NARROW SLOT) JUNCTION BOX MANHOLE RAFFIC BEARING DROP INL RAFFIC BEARING JUNCTIO BOX
THICKNESS OR GAUGE	FROM		ę P	FT	FT	FT	%			00	NOG NOG	.064	.064	920.	.109							*** R.C. PIPE *** RC PIPE (*** RC PIPE (15" SIDE DRA	18" SIDE DRA	CY	EACH TIN	H S	TT. TU.U. PA	TYPE OF GRA	DROP INLET	CATCH BASIN D.I. STD. 840	G.D.I. TYPE " T.B.D.I STD.	G.D.I. (N.S. FI	T.B.D.I STD.		CONC. & BRIC	CONC. COLL/ PIPE REMOV/		REMARKS
L 10+11 19 LT	0401			2694.4																					1							1	1					
	0401		0402		2692.5	2692.0		12			x x																											
L 11+50 25 LT	0403							56																														
TOTALS								68							Ī										1							1	1					

COMPUTED BY: DMB DATE:05/16/2025 CHECKED BY: DMB DATE: 05/16/2025

(9-17-24)

PROJECT NO. SHEET NO. DF18311.2095167.PR 3G-1

STATE OF NORTH CAROLINA **DIVISION OF HIGHWAYS**

SUIMMARY OF SUIBSUIRFACE DRAINAGE

LINE	Station	Station	Location LT/RT/CL	Drain Type* UD/BD/SD	LF
	CONTIN	IGENCY		SD	200
				TOTAL LF:	200

*UD = Underdrain

*BD = Blind Drain

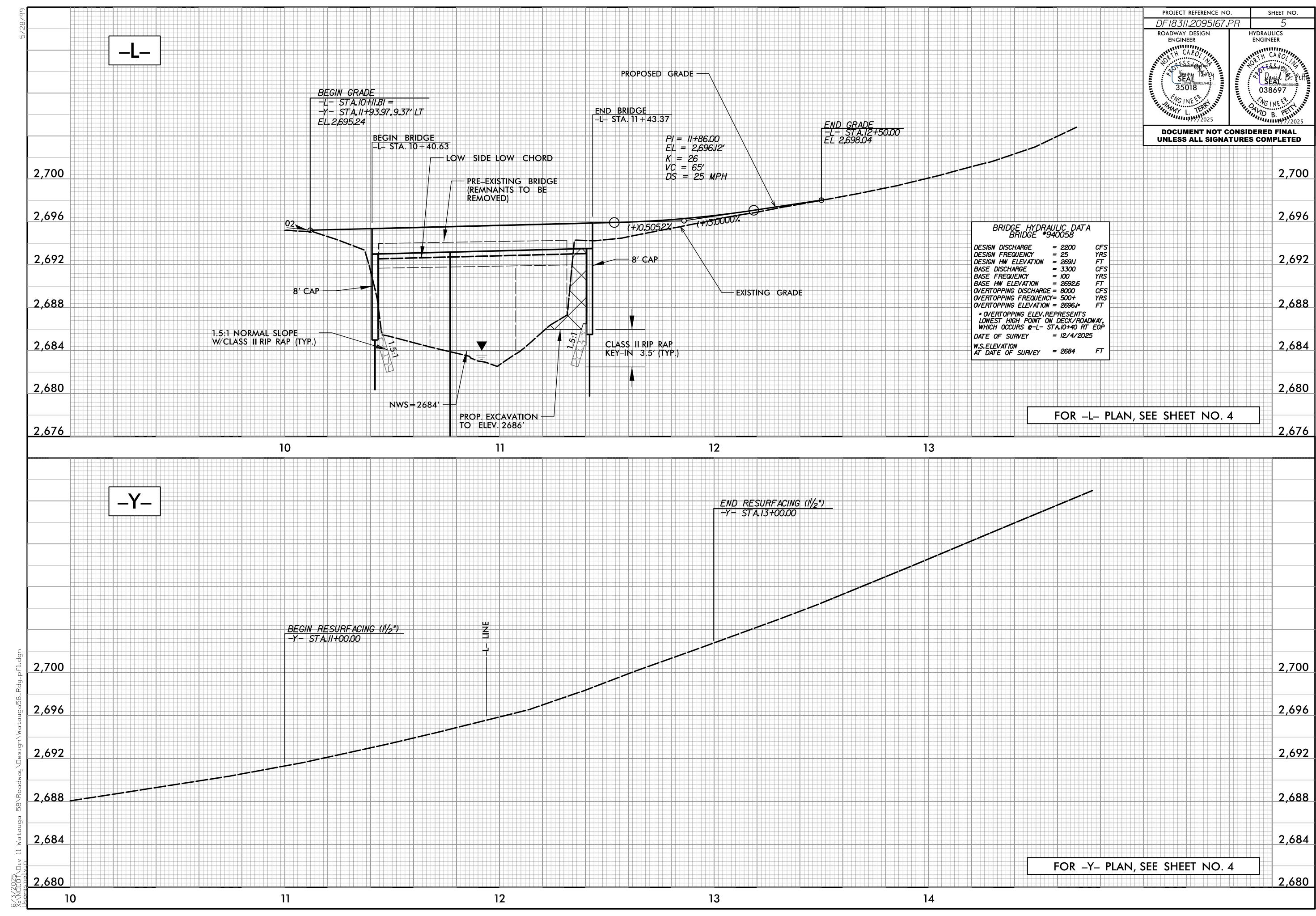
*SD = Subsurface Drain

SUMMARY OF AGGREGATE SUBGRADE/STABILIZATION

LINE	Station	Station	Aggregate Type* ASU(1/2)/ AST	Aggregate Thickness INCHES [8" for ASU(2)]	Shallow Undercut CY	Class IV Subgrade Stabilization TONS	Geotextile for Subgrade Stabilization SY	Stabilizer Aggregate TONS	Class IV Aggregate Stabilization TONS
	CONTINGENC	Y	ASU(1)	12	100	200	300		
			TOTAL	CY/TONS/SY:	100	200**	300**	0	0

^{*}ASU(1/2) = Aggregate Subgrade (Type 1 or 2)
*AST = Aggregate Stabilization

^{**}Total tons of "Class IV Subgrade Stabilization" and total square yards of "Geotextile for Subgrade Stabilization" are only the estimated quantities for ASU(1/2)/AST and may only represent a portion of the subgrade stabilization and geotextile quantities shown in the Item Sheets of the Proposal.



2095167 DF18311. PROJECT:

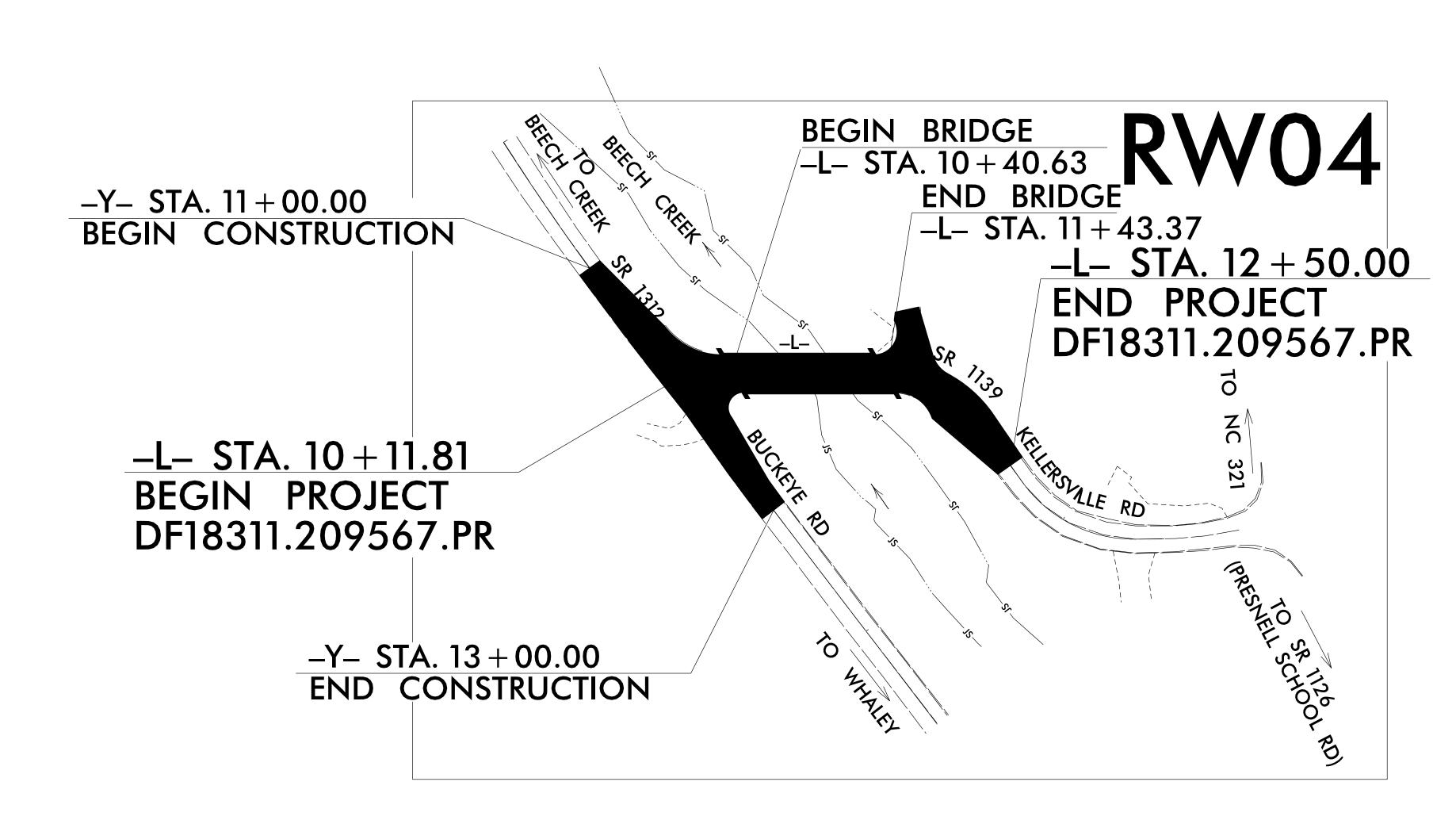
TIP

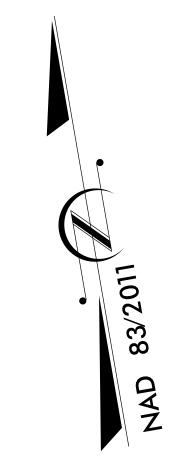
STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

N.C. DF18311.2095167.PR RW01 7

SURVEY CONTROL, EXISTING CENTERLINES, RIGHT OF WAY, EASEMENTS AND PROPERTY TIES

WATAUGA COUNTY





GRAPHIC SCALE PLANS

DATUM DESCRIPTION

THE LOCALIZED COORDINATE SYSTEM DEVELOPED FOR THIS PROJECT IS BASED ON THE STATE PLANE COORDINATES ESTABLISHED BY NCDOT FOR MONUMENT "GPS-1" WITH NAD 83/NSRS 2011 STATE PLANE GRID COORDINATES OF NORTHING: 920,556.47(ft) EASTING: 1,145,648.14(ft) **ELEVATION: 2,712.280(ft)** THE AVERAGE COMBINED GRID FACTOR USED ON THIS PROJECT (GROUND TO GRID) IS: 0.9998979435 THE N.C. LAMBERT GRID BEARING AND LOCALIZED HORIZONTAL GROUND DISTANCE FROM

"GPS-1" TO -L- STATION 10+00.00 IS

N 68°13'35.1" W 431.074(ft)

ALL LINEAR DIMENSIONS ARE LOCALIZED HORIZONTAL DISTANCES

VERTICAL DATUM USED IS NAVD 88

RIGHT OF WAY DATE:

3/28/2025

TGS ENGINEERS 20 | WEST MARION STREET SUITE 200 SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275 2024 STANDARD SPECIFICATIONS **LETTING DATE:**

08/21/2025

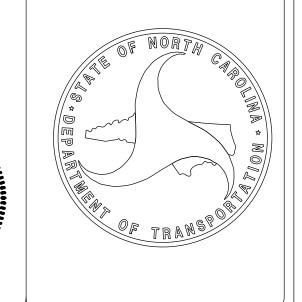
Prepared in the Office of:

Matthew Cornwell 6/9/2025 EBD36F11473E475.

PROFESSIONAL LAND

SURVEYOR

SIGNATURE:



-2025 07:57 veyors/projects/LIB\940 well at Mcornwelllaptop

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SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION



PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR RW02C-1

Location and Surveys

WSP USA Inc.
1001 WADE AVE
SUITE 400
PALEIGH, NC 27605
TEL: 919,830,4040
WWW.WSP.COM



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, Sarah J. Vincent, PLS, certify that the Project Control was performed under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: *AA*Type of GPS field procedure: RTN
Dates of survey: 10/23/2024-10/30/2024
Datum/Epoch: NAD 83/NSRS 2011
Published/Fixed-control use: N/A
Localized around: GPS-1
Northing: 920556.47
Easting: 1145648.14
Combined grid factor: 0.9998979435
Geoid model: GEOID18
Units: US Survey Feet

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed from 10/23/2024 to 10/30/2024, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 21st day of November, 2024.

SEE SHEET RW02C-3 FOR FURTHER ALIGNMENT DETAILS

NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

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SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

Location and	Surveys
DF18311.2095167.PR	RW02C-2
PROJECT REFERENCE NO.	SHEET N

WSP USA Inc. 1001 WADE AVE SUITE 400 RALEIGH, NC 27605 TEL: 919,836,4040 WWW.WSP.COM



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, Sarah J. Vincent, PLS, certify that the Project Control was performed under m
supervision from an actual GPS survey made under my supervision and the
following information was used to perform the survey:

Class of survey: AA
Type of GPS field procedure: RTN
Dates of survey: 10/23/2024-10/30/2024
Datum/Epoch: NAD 83/NSRS 2011
Published/Fixed-control use: N/A
Localized around: GPS-1
Northing: 920556.47
Easting: 1145648.14
Combined grid factor: 0.9998979435
Geoid model: GEOID18
Units: US Survey Feet

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed from 10/23/2024 to 10/30/2024, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 21st day of November, 2024.

Sarah Vincent

Professional Land Surveyor L-5617

3L					
	POINT	DESC.	NORTH	EAST	ELEVATION
 -					
2		GPS-2	920933.9600	1145555.0000	2731.06
1		GPS-1	920556.4700	1145648.1400	2712.28
4		BL - 4	920648.3600	1145461.5000	2696.80
3		BL - 3	920790.5200	1145224.9300	2690.38

BY				
POINT	DESC.	NORTH	EAST	ELEVATION
1 Ø 1	BY-101	920458.6300	1145368.8700	2716.20
3	BL - 3	920790.5200	1145224.9300	2690.38
102	BY-102	920980.4100	1145112.0500	2684.53

NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

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SURVEY CONTROL SHEET

W/EXISTING CENTERLINE ALIGNMENTS PRIOR TO CONSTRUCTION

Location and	Surveys
WSD	WSP USA Inc. 1001 WADE AVE SUITE 400 RALEGH, NC 27605 TEL: 919.836.4040 WWW.WSP.COM
SEAL L-5617	OR Note that the second
DOCUMENT NOT CONS UNLESS ALL SIGNATURE	

SHEET NO.
RW02C-3

PROJECT REFERENCE NO.

I, Sarah J. Vincent, PLS, certify that the Project Control was performed under my supervision from an actual GPS survey made under my supervision and the following information was used to perform the survey:

Class of survey: AA
Type of GPS field procedure: RTN
Dates of survey: 10/23/2024-10/30/2024
Datum/Epoch: NAD 83/NSRS 2011
Published/Fixed-control use: N/A
Localized around: GPS-1
Northing: 920556.47
Easting: 1145648.14
Combined grid factor: 0.9998979435
Geoid model: GEOID18
Units: US Survey Feet

I also certify that the Baseline Control for this project was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:20,000 (Class AA) and Vertical accuracy to Class A. Field work was performed from 10/23/2024 to 10/30/2024, and all coordinates are based on NAD 83/2011 and all elevations are based on NAVD 88; that this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

This 21st day of November, 2024.

Docusigned by:

Sarah Vincent

8605417B8A5848A...

Professional Land Surveyor L-5617

EL									
POINT	N	Е	BEARING	DIST	DELTA	D	L	T	R
OT	920716.371	1145247.820							
INE			S 80°31′04.0" E	132.34					
<u>'C</u>	920694.569	1145378.356							
CURVE			S 53°02′42.7" E	88.65	54°56′42.7"(RT)	59°37′51.1"	92.14	49.96	96.08
<u>'</u> T	920641.273	1145449.198							
INE			S 25°34′21.3" E	36.57					
<u>'C</u>	920608.289	1145464.982							
CURVE			S 53°12′53.Ø" E	82.79	55°17′Ø3.3"(LT)	64°13′07.9"	86.09	46.73	89.22
<u>'</u> T	920558.715	1145531.284							
INE			S 80°51′24.6" E	20.11					
C	920555.519	1145551.142							
CURVE			S 87°43′Ø9.5" E	59.74	13°43′29.7"(LT)	22°55′Ø5.9"	59.89	30.09	250.00
<u>'</u> T	920553.142	1145610.838							
INE			N 85°25′05.7" E	18.61					
OT	920554.628	1145629.385							

ΕY									
POINT	N	E	BEARING	DIST	DELTA		L	T	R
POT	920460.065	1145383.870							
LINE			N 27°57′35.Ø" W	330.56					
PC	920752.037	1145228.889							
CURVE			N 25°38′45.Ø" W	145.39	Ø4°37′4Ø.1"(RT)	Ø3°1Ø′56.1"	145.42	72.75	1800.47
PT	9201883 1010	11/5165 965							

NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE SURVEY CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

USLG729784 AT 42HYHW3

Docusign Envelope ID: 9E83A05A-1A81-4FE2-8F9E-39C6EC53E0A9 PROPOSED ALIGNMENT CONTROL SHEET

PROJECT REFERENCE NO.

DF18311.2095167.PR Location and Surveys



TGS ENGINEERS
20 | WEST MARION STREET SUITE 200 SHELBY, NC 28150 PH (704) 476-0003 CORP. LICENSE NO.: C-0275

SHEET NO.

PROJECT SURVEYOR

Matthew Cornwell

6/9/2025

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I, Matthew T. Cornwell, PLS, certify that the data compiled came from available surveys/mapping performed by others and provided to me by NCDOT and do not certify to the accuracy or quality of the individual data sources.

Matthew Cornwell FBD36F11473F475

Professional Land Surveyor L-4775

TYPE	STATION	NORTH	EAST
POT	10+00.00	920716.3725	1145247.8198
PC	11+43.31	920692.7627	1145389.1696
PT	12+15.23	920651.1616	1145444.4664
PC	12+62.35	920608.6581	1145464.8056
PT	13+49.19	920558.6505	1145531.6880
POT	13+68.89	920555.5195	1145551.1418

		Y	
TYPE	STATION	NORTH	EAST
PC	10+00.00	920884.0076	1145165.5755
PT	11+46.41	920752.0370	1145228.8888
POT	14+76.97	920460.0650	1145383.8700

NOTES:

- 1. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 2. THE PROPOSED ALIGNMENT CONTROL DATA FOR THIS PROJECT HAS BEEN COMPILED FROM VARIOUS SOURCES. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED, PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.

RIGHT OF WAY CONTROL SHEET

PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR RW03E-1

Location and Surveys

TGS ENGINEERS
201 WEST MARION STREET
SUITE 200
SHELBY, NC 28 150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

PROJECT SURVEYOR

Signed by:

Matthew Cornwell

EBD36F11473E475...

6/9/2025

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

I , Matthew T. Cornwell, certify that the right of way and permanent easement monumentation for this project shown herein was completed under my direct and responsible charge from an actual survey made under my supervision; that all horizontal closures had a minimum ratio of precision of 1:10,000 (Class A). Field work was performed from 6/4/2025 to 6/5/2025, and all coordinates are based on NAD83/2011; That this survey was performed to meet the requirements of 21NCAC 56.1600 as applicable.

6/9/2025

Signed by:

Matthew Cornwell

EBD36F11473E475...

Professional Land Surveyor L-4775

ROW MARKER IRON PIN AND CAP

ALIGN	STATION	OFFSET	NORTH	EAST			
Υ	11+46.79	-30.00	920765.7719	1145255.5626			

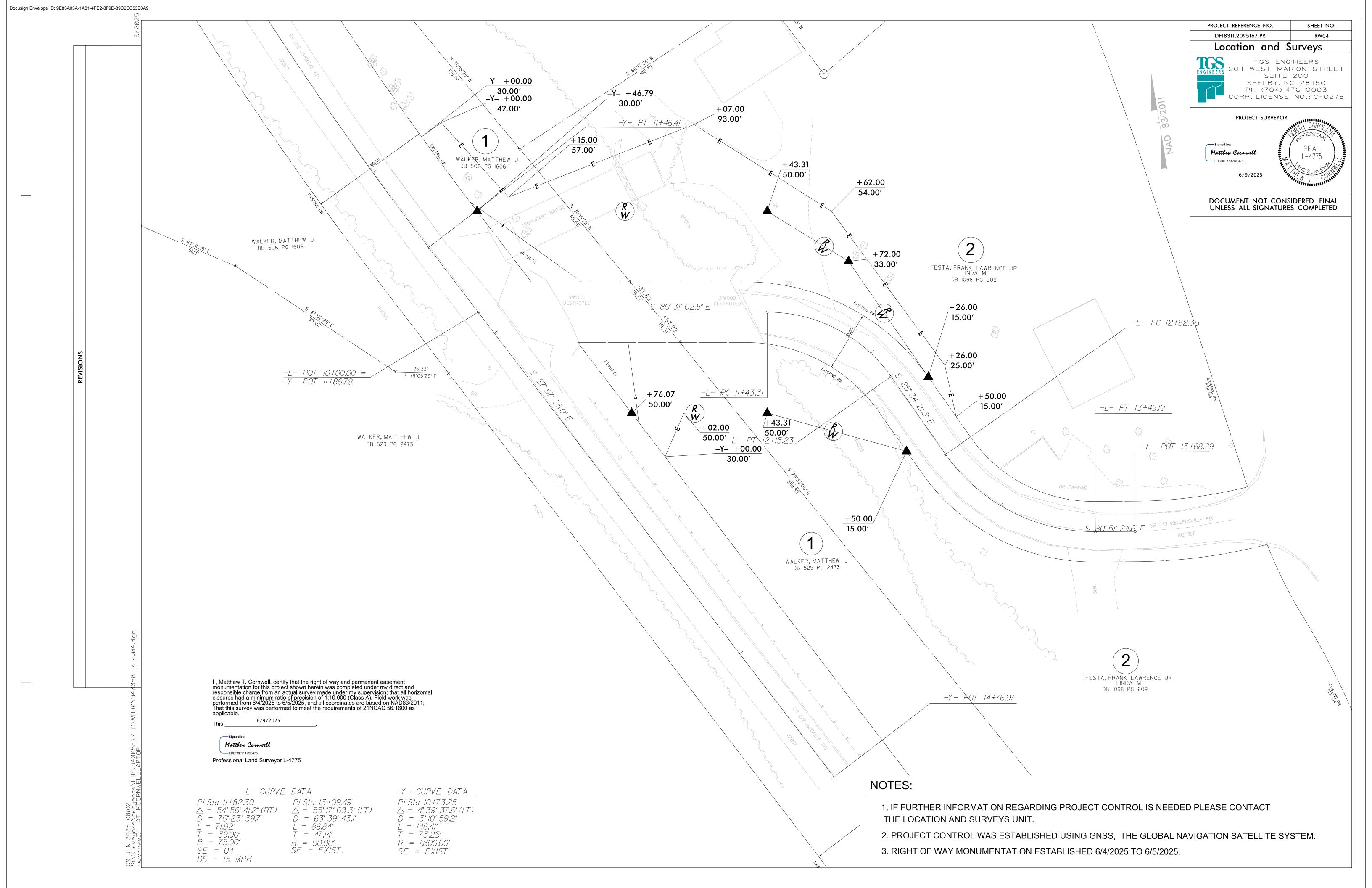
ROW MARKER IRON PIN AND CAP

	1 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \			
ALIGN	STATION	OFFSET	NORTH	EAST
L	10+76.07	50.00	920654.5231	1145314.6145
L	11+43.31	50.00	920643.4459	1145380.9321
L	11+43.31	-50.00	920742.0792	1145397.4090
L	11+72.00	-33.00	920710.9693	1145433.0852
L	12+26.00	-15.00	920647.9219	1145462.6456
L	12+50.00	15.00	920613.3234	1145445.9442

NOTES:

- 1. IF FURTHER INFORMATION REGARDING PROJECT CONTROL IS NEEDED PLEASE CONTACT THE LOCATION AND SURVEYS UNIT.
- 2. PROJECT CONTROL WAS ESTABLISHED USING GNSS, THE GLOBAL NAVIGATION SATELLITE SYSTEM.
- 3. RIGHT OF WAY MONUMENTATION ESTABLISHED 6/4/2025 TO 6/5/2025.

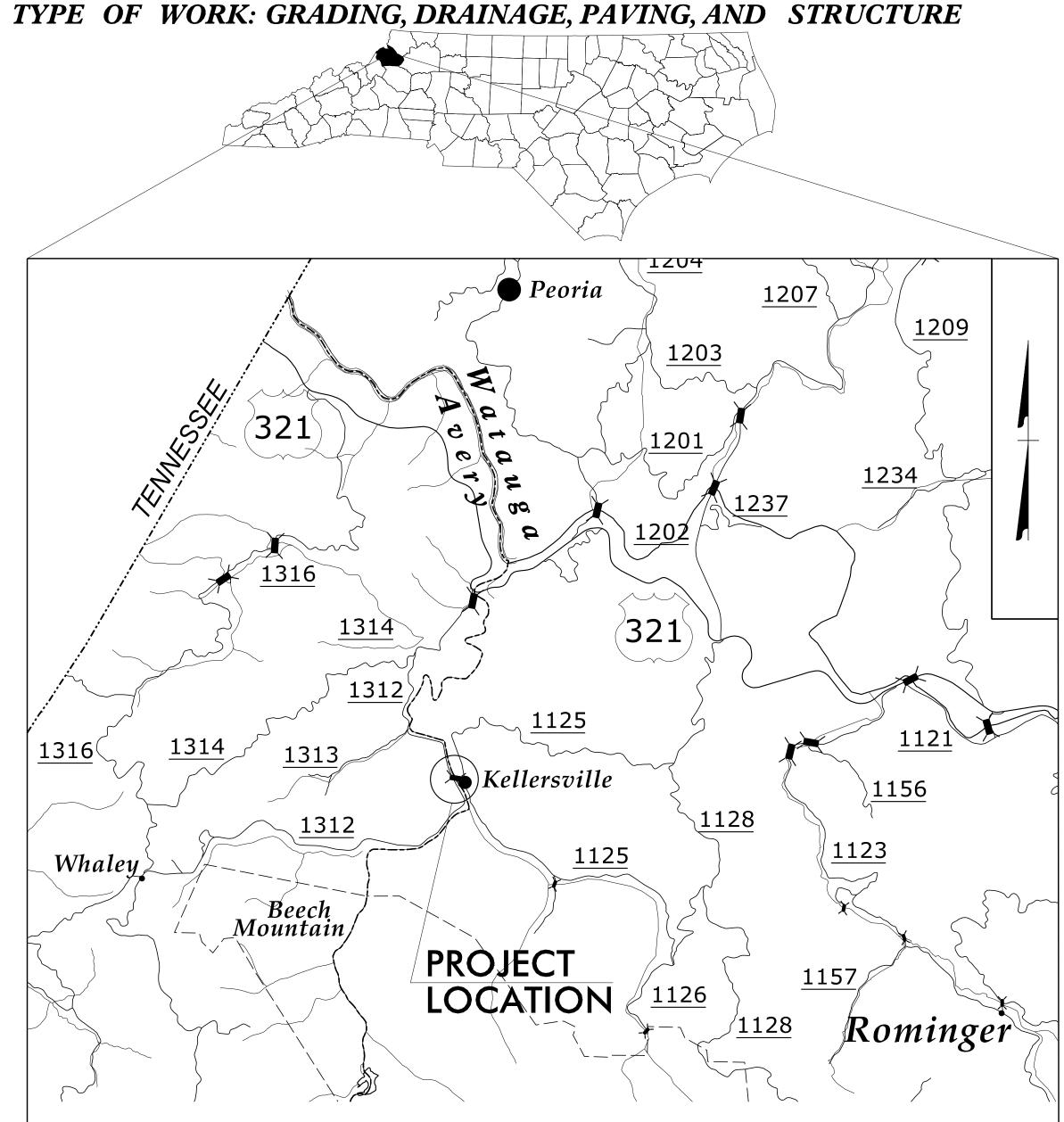
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TRANSPORTATION MANAGEMENT PLAN

WATAUGA COUNTY

LOCATION: BRIDGE #940058 OVER BEECH CREEK ON SR 1139 (KELLERSVILLE RD)



VICINITY MAP



PLANS PREPARED FOR N.C.D.O.T. BY: TGS ENGINEERS



TGS ENGINEERS 706 HILLSBOROUGH ST. SUITE 200 RALEIGH, NC 27603 PH (919) 773-8887 CORP. LICENSE NO.: C-0275



CODA BRANNAN, E.I. DESIGN ENGINEER



INDEX OF SHEETS

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TMP-3 TEMPORARY TRAFFIC CONTROL PHASING TMP-4 THRU TEMPORARY TRAFFIC CONTROL PHASE I

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TMP-6 TEMPORARY TRAFFIC CONTROL PHASE II

> **DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED**

APPROVED: Don A. Parker
75DB9E90ADEF440... DATE:_

SEAL



TMP-1

SHEET NO.

2095167 8311

ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2024 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.	TITLE
1101.01	WORK ZONE WARNING SIGNS
1101.02	TEMPORARY LANE CLOSURES
1101.11	TRAFFIC CONTROL DESIGN TABLES
1110.01	STATIONARY WORK ZONE SIGNS
1110.02	PORTABLE WORK ZONE SIGNS
1130.01	DRUMS
1135.01	CONES
1145.01	BARRICADES
1150.01	FLAGGERS
1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO LANE AND MULTILANE ROADWAYS
1205.04	PAVEMENT MARKINGS - INTERSECTIONS
1205.12	PAVEMENT MARKINGS - BRIDGES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING
1262.01	GUARDRAIL END DELINEATION

DF183111.2095167.PR TMP-1A TGS ENGINEERS 706 HILLSBOROUGH ST. SUITE 200 RALEIGH, NC 27603 PH (919) 773-8887 CORP. LICENSE NO.: C-0275

PROJ. REFERENCE NO. SHEET NO.

LEGEND

GENERAL

DIRECTION OF TRAFFIC FLOW

DIRECTION OF PEDESTRIAN TRAFFIC FLOW

----- EXIST. PVMT.

NORTH ARROW

PROPOSED PVMT.

TEMP. SHORING (LOCATION PURPOSES ONLY)

WORK AREA

REMOVAL

SIGNALS

EXISTING

PAVEMENT MARKINGS

——EXISTING LINES ——TEMPORARY LINES

TRAFFIC CONTROL DEVICES

BARRICADE (TYPE III)

DRUM SKINNY DRUM O TUBULAR MARKER

TEMPORARY CRASH CUSHION

FLASHING ARROW BOARD

FLAGGER

LAW ENFORCEMENT

TRUCK MOUNTED ATTENUATOR (TMA)

CHANGEABLE MESSAGE SIGN

TEMPORARY SIGNING

PORTABLE SIGN

── STATIONARY SIGN

STATIONARY OR PORTABLE SIGN

PAVEMENT MARKERS

CRYSTAL/CRYSTAL

CRYSTAL/RED YELLOW/YELLOW

PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

TEMPORARY PAVEMENT MARKING

SYMBOL

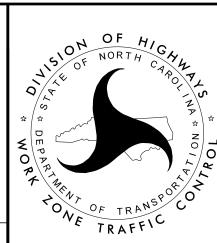
DESCRIPTION

P61

WHITE STOPBAR (24")

Don A. Parker APPROVED: _ DOCUMENT NOT CONSIDERED FINAL

UNLESS ALL SIGNATURES COMPLETED



ROADWAY STANDARD DRAWINGS & LEGEND

CHANGES MAY BE REQUIRED WHEN PHYSICAL DIMENSIONS IN THE DETAIL DRAWINGS, STANDARD DETAILS, AND ROADWAY DETAILS ARE NOT ATTAINABLE TO MEET FIELD CONDITIONS OR RESULT IN DUPLICATE OR UNDESIRED OVERLAPPING OF DEVICES. MODIFICATION MAY INCLUDE: MOVING, SUPPLEMENTING, COVERING, OR REMOVAL OF DEVICES AS DIRECTED BY THE ENGINEER.

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT EXCEPT WHEN OTHERWISE NOTED IN THE PLAN OR DIRECTED BY THE ENGINEER.

LANE AND SHOULDER CLOSURE REQUIREMENTS

- A) REMOVE LANE CLOSURE DEVICES FROM THE LANE WHEN WORK IS NOT BEING PERFORMED BEHIND THE LANE CLOSURE OR WHEN A LANE CLOSURE IS NO LONGER NEEDED OR AS DIRECTED BY THE ENGINEER.
- B) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 15 FT OF AN OPEN TRAVEL LANE, CLOSE THE NEAREST OPEN SHOULDER USING ROADWAY STANDARD DRAWING NO. 1101.04 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL OR A LANE CLOSURE IS INSTALLED.
- C) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 5 FT OF AN OPEN TRAVEL LANE ON AN UNDIVIDED FACILITY, CLOSE THE NEAREST OPEN TRAVEL LANE USING ROADWAY STANDARD DRAWING NO. 1101.02 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL.

WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN 10 FT OF AN OPEN TRAVEL LANE ON A DIVIDED FACILITY, CLOSE THE NEAREST OPEN TRAVEL LANE USING ROADWAY STANDARD DRAWING NO. 1101.02 UNLESS THE WORK AREA IS PROTECTED BY BARRIER OR GUARDRAIL.

- D) WHEN PERSONNEL AND/OR EQUIPMENT ARE WORKING WITHIN A LANE OF TRAVEL OF AN UNDIVIDED OR DIVIDED FACILITY, CLOSE THE LANE ACCORDING TO THE TRAFFIC CONTROL PLANS, ROADWAY STANDARD DRAWINGS, OR AS DIRECTED BY THE ENGINEER. CONDUCT THE WORK SO THAT ALL PERSONNEL AND/OR EQUIPMENT REMAIN WITHIN THE CLOSED TRAVEL LANE.
- E) DO NOT WORK SIMULTANEOUSLY WITHIN 15 FT ON BOTH SIDES OF AN OPEN TRAVELWAY, RAMP, OR LOOP WITHIN THE SAME LOCATION UNLESS PROTECTED WITH GUARDRAIL OR BARRIER.

PAVEMENT EDGE DROP OFF REQUIREMENTS

F) BACKFILL AT A 6:1 SLOPE UP TO THE EDGE AND ELEVATION OF EXISTING PAVEMENT IN AREAS ADJACENT TO AN OPENED TRAVEL LANE THAT HAS AN EDGE OF PAVEMENT DROP-OFF AS FOLLOWS:

BACKFILL DROP-OFFS THAT EXCEED 2 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS OF 45 MPH OR GREATER.

BACKFILL DROP-OFFS THAT EXCEED 3 INCHES ON ROADWAYS WITH POSTED SPEED LIMITS LESS THAN 45 MPH.

BACKFILL WITH SUITABLE COMPACTED MATERIAL, AS APPROVED BY THE ENGINEER, AT NO EXPENSE TO THE DEPARTMENT.

TRAFFIC PATTERN ALTERATIONS

G) NOTIFY THE ENGINEER THIRTY (30) CALENDAR DAYS PRIOR TO ANY TRAFFIC PATTERN ALTERATION.

SIGNING

- H) INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.
- I) ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.
- J) INSTALL BLACK ON ORANGE "DIP" SIGNS (W8-2) AND/OR "BUMP" SIGNS (W8-1) 100 FT IN ADVANCE OF THE UNEVEN AREA, OR AS DIRECTED BY THE ENGINEER.

TRAFFIC BARRIER

K) INSTALL TEMPORARY BARRIER ACCORDING TO THE TRANSPORTATION MANAGEMENT PLANS A MAXIMUM OF TWO (2) WEEKS PRIOR TO BEGINNING WORK IN ANY LOCATION. ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION PROCEED IN A CONTINUOUS MANNER TO COMPLETE THE PROPOSED WORK IN THAT LOCATION UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS OR AS DIRECTED BY THE ENGINEER.

DO NOT PLACE BARRIER DIRECTLY ON ANY SURFACE OTHER THAN ASPHALT OR CONCRETE.

ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION AND NO WORK IS PERFORMED BEHIND THE TEMPORARY BARRIER FOR A PERIOD LONGER THAN TWO (2) MONTHS, REMOVE / RESET TEMPORARY BARRIER AT NO COST TO THE DEPARTMENT UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS, TEMPORARY BARRIER IS PROTECTING A HAZARD, OR AS DIRECTED BY THE ENGINEER.

INSTALL TEMPORARY BARRIER WITH THE TRAFFIC FLOW BEGINNING WITH THE UPSTREAM SIDE OF TRAFFIC. REMOVE TEMPORARY BARRIER AGAINST THE TRAFFIC FLOW BEGINNING WITH THE DOWNSTREAM SIDE OF TRAFFIC.

INSTALL AND SPACE DRUMS NO GREATER THAN TWICE THE POSTED SPEED LIMIT (MPH) TO CLOSE OR KEEP THE SECTION OF THE ROADWAY CLOSED UNTIL THE TEMPORARY BARRIER CAN BE PLACED OR AFTER THE TEMPORARY BARRIER IS REMOVED.

L) PROTECT THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER AT ALL TIMES DURING THE INSTALLATION AND REMOVAL OF THE BARRIER BY EITHER A TRUCK MOUNTED ATTENUATOR (MAXIMUM 72 HOURS) OR A TEMPORARY CRASH CUSHION.

PROTECT THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER FROM ONCOMING TRAFFIC AT ALL TIMES BY A TEMPORARY CRASH CUSHION UNLESS THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER IS OFFSET FROM ONCOMING TRAFFIC AS FOLLOWS OR AS SHOWN IN THE PLANS: (SEE ALSO 1101.05)

POSTED SPEED LIMIT	MINIMUM OFFSET
40 OR LESS	15 FT
45 - 50	20 FT
55	25 FT
60 MPH or HIGHER	30 FT

TRAFFIC CONTROL DEVICES

- M) WHEN LANE CLOSURES ARE NOT IN EFFECT SPACE CHANNELIZING DEVICES IN WORK AREAS NO GREATER IN FEET THAN TWICE THE POSTED SPEED LIMIT (MPH) EXCEPT, 10 FT ON-CENTER IN RADII, AND 3 FT OFF THE EDGE OF AN OPEN TRAVELWAY. REFER TO STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES SECTIONS 1130 (DRUMS), 1135 (CONES) AND 1180 (SKINNY DRUMS) FOR ADDITIONAL REQUIREMENTS.
- N) PLACE TYPE III BARRICADES, WITH "ROAD CLOSED" SIGN R11-2 ATTACHED. OF SUFFICIENT LENGTH TO CLOSE ENTIRE ROADWAY.

PAVEMENT MARKINGS AND MARKERS

O) INSTALL TEMPORARY PAVEMENT MARKINGS AND TEMPORARY PAVEMENT MARKERS ON INTERIM LAYERS OF PAVEMENT AS FOLLOWS:

ROAD NAME		MARKING	MARKER	
- L -	SR 1139	KELLERSVILLE RD	PAINT	NONE
- Y -	SR 1312	BUCKEYE RD	PAINT	NONE

- P) PLACE ONE APPLICATION OF PAINT FOR TEMPORARY TRAFFIC PATTERNS. PLACE A SECOND APPLICATION OF PAINT SIX (6) MONTHS AFTER THE INITIAL APPLICATION AND EVERY SIX MONTHS AS DIRECTED BY THE ENGINEER.
- Q) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.
- R) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS BY THE END OF EACH DAY'S OPERATION.

PROJ. RE	PROJ. REFERENCE NO.	
DF18311	.2095167.PR	TMP-1B
TES	706 HILLSBOROL RALEIGH	GINEERS JGH ST. SUITE 200 J, NC 27603 J 773-8887
		SE NO.: C-0275

MANAGEMENT STRATEGIES

THE FOLLOWING LISTED WORK ZONE STRATEGIES ARE RECOMMENDED FOR INCLUSION WITHIN THIS TRANSPORTATION MANAGEMENT PLAN (TMP).

RECOMMENDED STRATEGIES:

TRAFFIC MANAGEMENT STRATEGIES:

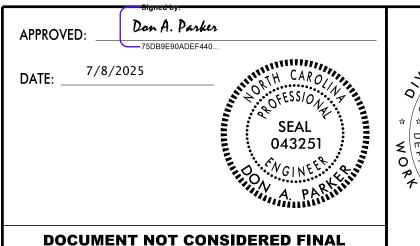
LANE SHIFTS OR CLOSURES

SHOULDER CLOSURES

ONE-LANE, TWO WAY OPERATION (FLAGGING)
ONE-LANE. TWO WAY OPERATION (SIGNALIZED)

LOCAL NOTES

DUE TO SITE CONDITIONS, PLACEMENT OF THE WATERFILLED BARRIER FROM -L- STA. 11+34± TO STA. 11+56± IS NOT REQUIRED TO BE ON AN ASPHALT OR CONCRETE SURFACE.



UNLESS ALL SIGNATURES COMPLETED



TRANSPORTATION
OPERATIONS
PLAN

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FIGURE A

NOTES

- 1- REFER TO THE TRAFFIC CONTROL PLANS FOR TEMPORARY SHORING LOCATIONS AND NOTES.
- 2- REFER TO THE "TEMPORARY SHORING" STANDARD PROVISION FOR INFORMATION ABOUT TEMPORARY SHORING AND PORTABLE CONCRETE BARRIER (PCB).
- 3- PCB IS REQUIRED IF TEMPORARY SHORING/WALL IS LOCATED WITHIN THE CLEAR ZONE IN ACCORDANCE WITH THE AASHTO ROADSIDE DESIGN GUIDE. DO NOT PLACE BARRIER DIRECTLY ON ANY SURFACE OTHER THAN ASPHALT OR CONCRETE.

 (CONTACT NCDOT PAVEMENT MANAGEMENT FOR APPLICABLE PAVEMENT DESIGN).
- 4- BASED ON THE CLEAR DISTANCE, OFFSET, DESIGN SPEED AND PAVEMENT TYPE, CHOOSE AN UNANCHORED OR ANCHORED PCB FROM THE TABLE SHOWN IN FIGURE B. CLEAR DISTANCE IS DEFINED AS SHOWN IN FIGURE A AND OFFSET IS DEFINED AS SHOWN IN FIGURE B.
- 5- AT THE CONTRACTOR'S OPTION OR IF THE MINIMUM REQUIRED CLEAR DISTANCE IS NOT AVAILABLE, SET PCB NEXT TO AND UP AGAINST THE TRAFFIC SIDE OF THE TEMPORARY SHORING/WALLS EXCEPT FOR BARRIER ABOVE TEMPORARY WALLS. PCB WITH THE MINIMUM REQUIRED CLEAR DISTANCE IS REQUIRED ABOVE TEMPORARY WALLS.
- 6- USE NCDOT PORTABLE CONCRETE BARRIER (PCB) IN ACCORDANCE WITH ROADWAY STANDARD DRAWING NO. 1170.01 AND SECTION 1170 OF THE STANDARD SPECIFICATIONS.
- 7- SET PCB WITH A MINIMUM HORIZONTAL DISTANCE OF 2 FT BETWEEN THE FRONT FACE OF THE BARRIER AND THE EDGE OF THE NEAREST TRAFFIC LANE AS SHOWN IN FIGURE A UNLESS OTHERWISE SHOWN IN THE PLANS OR APPROVED BY THE ENGINEER.
- 8- FOR PCB ABOVE AND BEHIND TEMPORARY WALLS, PROVIDE A MINIMUM DISTANCE OF 3 FT BETWEEN THE EDGE OF PAVEMENT AND THE WALL FACE AS SHOWN IN FIGURE A. IF THIS MINIMUM REQUIRED DISTANCE IS NOT AVAILABLE, CONTACT THE ENGINEER.
- 9- TABLE SHOWN IN FIGURE B IS BASED ON NCDOT RESEARCH PROJECT NO. 2005-010 WITH VEHICLE TYPE USED FOR NCHRP 350 CRASH TESTS.

PROJ. REFERENCE NO.	SHEET NO.
DF18311.2095167.PR	TMP-2

MINIMUM REQUIRED CLEAR DISTANCE, inches

Barrier	Pavement	Offset *			sign Spe			
Type	Type	ft	<30	31-40	41-50	51-60	61-70	71-80
V 1		<8	24	26	29	32	36	40
		8-14	26	28	31	35	38	42
Type Type ft <8 8-14 14-20 20-26 26-32 32-38 38-44 44-50 50-56 <8 8-14 14-20 20-26 <8 8-14 14-20 20-26 <8 32-38 38-44 44-50 50-56 >56 <8 38-44 44-50 50-56 >56 <8 Solution Solut	14-20	27	29	34	36	39	43	
		20-26	28	31	35	38	40	44
	Asnhalt	26-32	29	32	36	39	42	45
	risphare	32-38	30	34	38	41	43	46
Ą		38-44	31	34	41	43	45	48
PC		44-50	31	35	41	43	46	49
		50-56	32	36	42	44	47	50
re		>56	32	36	42	45	47	51
\mathbf{h}_0		<8	17	18	21	22	25	26
nc		8-14	19	20	23	25	26	29
na		14-20	22	22	24	26	28	31
n		20-26	23	24	26	27	30	34
	Concrete	26-32	24	25	27	28	32	35
		32-38	24	26	27	30	33	36
		38-44	25	26	28	30	34	37
		44-50	26	26	28	32	35	37
		50-56	26	26	28	32	35	38
		>56	26	27	29	32	36	38
4	Asphalt			24 f	or All D	esign Sp	eeds	
Anchored PCB	Concrete (including bridge approach slabs)	All Offsets		12 f	or All D	esign Sp	eeds	

* See Figure Below

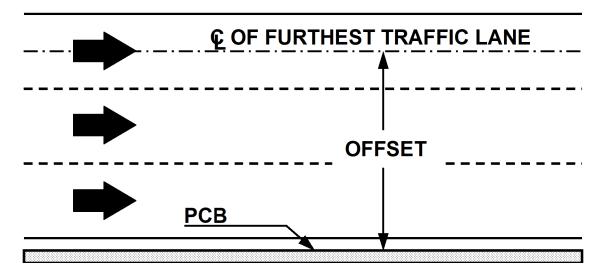
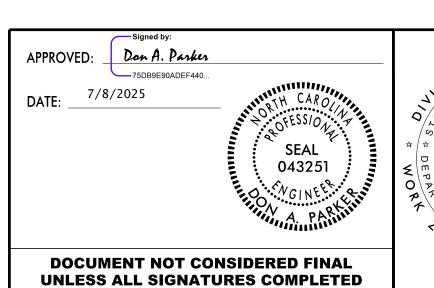
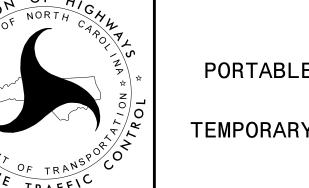


FIGURE B





PORTABLE CONCRETE BARRIER
AT
TEMPORARY SHORING LOCATIONS

SHORING NOTES

PROJ. REFERENCE NO.

DF18311.2095167.PR

TMP - 2A

TGS ENGINEERS
706 HILLSBOROUGH ST. SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275

Shoring Location No. 1 (CUT SHORING):

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE STRUCTURE CONSTRUCTION FROM -L- STATION 10+17±, 19.7' LT TO -L- STATION 10+34±, 19.7' LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM -L- STATION 10+17±, 19.7' LT TO -L- STATION 10+34±, 19.7' LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS:

ABOVE ELEVATION 2,681.3 FT UNIT WEIGHT (γ) = 120 LB/CF FRICTION ANGLE (ϕ) = 26 DEGREES COHESION (c) = 0 LB/SF

ELEVATION 2,681.3 FT TO ELEVATION 2,679.5 FT UNIT WEIGHT (γ) = 135 LB/CF FRICTION ANGLE (ϕ) = 38 DEGREES COHESION (c) = 500 LB/SF GROUNDWATER ELEVATION = 2,681.3 FT

BELOW ELEVATION 2,679.5 FT (ROCK) UNIT WEIGHT (γ) = 165 LB/CF FRICTION ANGLE (ϕ) = 40 DEGREES COHESION (c) = 1000 LB/SF

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM -L- STATION 10+17±, 19.7' LT TO -L- STATION 10+34±, 19.7' LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

DRIVEN PILING FOR TEMPORARY SHORING FROM -L- STATION 10+17±, 19.7′ LT TO -L- STATION 10+34±, 19.7′ LT MAY NOT PENETRATE BELOW ELEVATION 2,681.3 FT DUE TO OBSTRUCTION, VERY DENSE OR HARD SOIL, BOULDERS, OR WEATHERED OR HARD ROCK.

DO NOT USE STANDARD TEMPORARY SHORING FOR TEMPORARY SHORING -L-STATION 10+17±, 19.7' LT TO -L-STATION 10+34±, 19.7' LT. CONTRACTOR DESIGNED SHORING IS REQUIRED. SEE TEMPORARY SHORING SPECIAL PROVISION.

IF GROUNDWATER OR THE FLOOD ELEVATION IS ABOVE THE BOTTOM OF THE REINFORCED ZONE, DO NOT USE A TEMPORARY WALL FOR TEMPORARY SHORING FROM -L- STATION 10+17±, 19.7' LT TO -L- STATION 10+34±, 19.7' LT.

Shoring Location No. 2 (CUT SHORING):

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE STRUCTURE CONSTRUCTION FROM -Y- STATION 11+81±, 25.3' LT TO -Y- STATION 12+37±, 18.2' LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM -Y- STATION 11+81±, 25.3' LT TO -Y- STATION 12+37±, 18.2' LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS:

ABOVE ELEVATION 2,688.2 FT UNIT WEIGHT (γ) = 120 LB/CF FRICTION ANGLE (ϕ) = 26 DEGREES COHESION (c) = 0 LB/SF

ELEVATION 2,688.2 FT TO ELEVATION 2,682.8 FT UNIT WEIGHT (γ) = 120 LB/CF FRICTION ANGLE (ϕ) = 30 DEGREES COHESION (c) = 0 LB/SF GROUNDWAY ELEVATION = 2,687.7

BELOW ELEVATION 2,682.8 FT (ROCK) UNIT WEIGHT (γ) = 165 LB/CF FRICTION ANGLE (ϕ) = 40 DEGREES COHESION (c) = 1000 LB/SF

LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM -Y- STATION 11+81±, 25.3' LT TO -Y- STATION 12+37±, 18.2' LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

DRIVEN PILING FOR TEMPORARY SHORING FROM -Y- STATION 11+81±, 25.3′ LT TO -Y- STATION 12+37±, 18.2′ LT MAY NOT PENETRATE BELOW ELEVATION 2,682.8 FT DUE TO OBSTRUCTION, VERY DENSE OR HARD SOIL, BOULDERS, OR WEATHERED OR HARD ROCK.

DO NOT USE STANDARD TEMPORARY SHORING FOR TEMPORARY SHORING FROM -Y- STATION 11+81±, 25.3' LT TO -Y- STATION 12+37±, 18.2' LT. CONTRACTOR DESIGNED SHORING IS REQUIRED. SEE TEMPORARY SHORING SPECIAL PROVISION.

DO NOT USE A TEMPORARY WALL FOR TEMPORARY SHORING FROM -Y-STATION 11+81±, 25.3' LT TO -Y-STATION 12+37±, 18.2' LT.

Shoring Location No. 3 (CUT SHORING):

FOR TEMPORARY SHORING AND POSITIVE PROTECTION FOR TEMPORARY SHORING, SEE PLANS AND TEMPORARY SHORING PROVISION.

TEMPORARY SHORING IS REQUIRED FOR THE STRUCTURE CONSTRUCTION FROM -L- STATION 11+41±, 25.0' LT TO -L- STATION 11+79±, 17.5' LT.

BEFORE BEGINNING TEMPORARY SHORING DESIGN OR CONSTRUCTION, SURVEY EXISTING GROUND ELEVATIONS IN THE VICINITY OF SHORING LOCATIONS TO DETERMINE ACTUAL SHORING HEIGHTS.

DESIGN TEMPORARY SHORING FROM -L- STATION 11+41±, 25.0' LT TO -L- STATION 11+79±, 17.5' LT, FOR THE FOLLOWING ASSUMED SOIL PARAMETERS:

ABOVE ELEVATION 2,670.3 FT UNIT WEIGHT (γ) = 120 LB/CF FRICTION ANGLE (ϕ) = 26 DEGREES COHESION (c) = 0 LB/SF GROUNDWATER ELEVATION = 2,687.0 FT

ELEVATION 2,670.3 FT TO ELEVATION 2,664.9 FT UNIT WEIGHT (γ) = 135 LB/CF FRICTION ANGLE (ϕ) = 38 DEGREES COHESION (c) = 500 LB/SF

BELOW ELEVATION 2,663.7 FT (ROCK) UNIT WEIGHT (γ) = 165 LB/CF FRICTION ANGLE (ϕ) = 40 DEGREES COHESION (c) = 1000 LB/SF

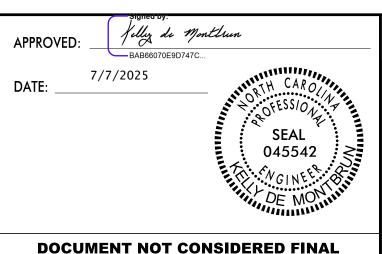
LIMITED SUBSURFACE INFORMATION IS AVAILABLE IN THE VICINITY OF TEMPORARY SHORING FROM -L- STATION 11+41±, 25.0' LT TO -L- STATION 11+79±, 17.5' LT. THE INFORMATION PROVIDED FOR TEMPORARY SHORING DESIGN WAS ASSUMED AND MAY NOT BE APPLICABLE TO THE ACTUAL CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

DRIVEN PILING FOR TEMPORARY SHORING FROM -L- STATION 11+41±, 25.0′ LT TO -L- STATION 11+79±, 17.5′ LT MAY NOT PENETRATE BELOW ELEVATION 2,684.9 FT DUE TO OBSTRUCTION, VERY DENSE OR HARD SOIL, BOULDERS, OR WEATHERED OR HARD ROCK.

DO NOT USE STANDARD TEMPORARY SHORING FOR TEMPORARY SHORING FROM -L- STATION 11+41±, 25.0' LT TO -L- STATION 11+79±, 17.5' LT. CONTRACTOR DESIGNED SHORING IS REQUIRED. SEE TEMPORARY SHORING SPECIAL PROVISION.

DO NOT USE A TEMPORARY WALL FOR TEMPORARY SHORING FROM -L-STATION 11+41±. 25.0' LT TO -LSTATION 11+79±. 17.5' LT.

)]\Div || Watauga 58\Work Zone Traffic Control\Watauga58



UNLESS ALL SIGNATURES COMPLETED



PHASING

PROJ. REFERENCE NO.

DF18311.2095167.PR

TMP - 3

TGS ENGINEERS
706 HILLSBOROUGH ST. SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275

NOTE: FOR ALL FLAGGING OPERATIONS, SEE RSD 1101.02, SHEET 1.

PHASE I

STEP 1 -- INSTALL WORK ZONE ADVANCE WARNING SIGNS IN ACCORDANCE WITH RSD 1101.01, SHEET 3 AND TMP-4.

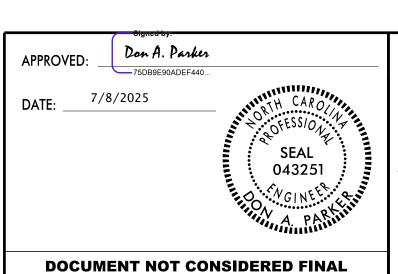
- STEP 2 -- INSTALL TEMPORARY PORTABLE SIGNAL AND WARNING SIGNS AND SHIFT TRAFFIC TO THE PHASE I SIGNALIZED PATTERN (SEE TMP-4 AND SPECIAL PROVISION).
 - -- INSTALL WATERFILLED BARRIER AS DETAILED ON TMP-4 (SEE LOCAL NOTES).
- STEP 3 -- AWAY FROM TRAFFIC PERFORM THE FOLLOWING (SEE TMP-4):
 - -- INSTALL TEMPORARY SHORING 1, 2, AND 3. (SEE TMP-2,2A, AND 4).
 - -- CONSTRUCT PROPOSED BRIDGE AND APPROACH SLABS FROM -L- STA. 10+30 +/- TO -TAN EXT- STA. 11+54 +/-.
- STEP 4 -- AWAY FROM TRAFFIC REMOVE TEMPORARY SHORING 1, 2, AND 3.
- STEP 5 -- USING FLAGGERS, REMOVE AND STOCKPILE WATERFILLED BARRIER AND REPLACE WITH DRUMS (SEE TMP-5).
- STEP 6 -- UNDER SIGNAL CONTROL, AND USING FLAGGERS AS NECESSARY, PERFORM THE FOLLOWING (SEE TMP-5):
 - -- CONSTRUCT FULL DEPTH ROADWAY FROM -L- STA 10+12 +/TO -L- STA. 10+30 +/- INCLUDING PAVED SHOULDER TO
 -Y- STA. 12+82 +/-, UP TO BUT NOT INCLUDING FINAL
 LAYER OF SURFACE COURSE.
 - -- CONSTRUCT FULL DEPTH ROADWAY INCLUDING PAVED SHOULDER FROM -TAN EXT- STA 11+54 +/- TO -L- STA. 12+50 +/- (RIGHT LANE), UP TO BUT NOT INCLUDING FINAL LAYER OF SURFACE COURSE.
 - -- INSTALL PROPOSED RIGHT SIDE GUARDRAIL.
 - -- PLACE ASPHALT OVERLAY ON PROPOSED BRIDGE

PHASE II

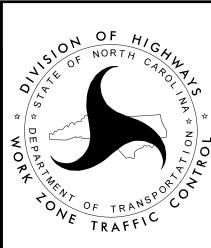
- STEP 1 -- USING FLAGGERS, SHIFT 1 LANE-2 WAY TRAFFIC ON KELLERSVILLE RD. (-L-) INTO THE RIGHT LANE AND ONTO THE PROPOSED BRIDGE (SEE TMP-6).
 - -- USE TYPE III BARRICADES TO CLOSE ACCESS TO TEMPORARY BRIDGE.
- STEP 2 -- RESET WATERFILLED BARRIER TO -Y- STA. 11+00 +/- TO -L- STA. 10+79 +/- (SEE TMP-6).
- STEP 3 -- UNDER SIGNAL CONTROL, AND USING FLAGGERS AS NECESSARY, PERFORM THE FOLLOWING (SEE TMP-6):
 - -- REMOVE TEMPORARY BRIDGE.
 - -- CONSTRUCT FULL DEPTH ROADWAY INCLUDING DRIVE AND PIPE TO PARCEL 1 FROM -TAN EXT- STA 11+54 +/- TO -L- STA. 12+50 +/- (LEFT LANE), UP TO BUT NOT INCLUDING FINAL LAYER OF SURFACE COURSE.
 - -- CONSTRUCT PAVED SHOULDER INCLUDING DRAINAGE STRUCTURE 401 AND 402 FROM -Y-STA. 11+00 +/- TO -Y-STA. 11+93 +/- UP TO BUT NOT INCLUDING THE FINAL LAYER OF SURFACE COURSE.
 - -- INSTALL PROPOSED LEFT SIDE GUARDRAIL

PHASE III

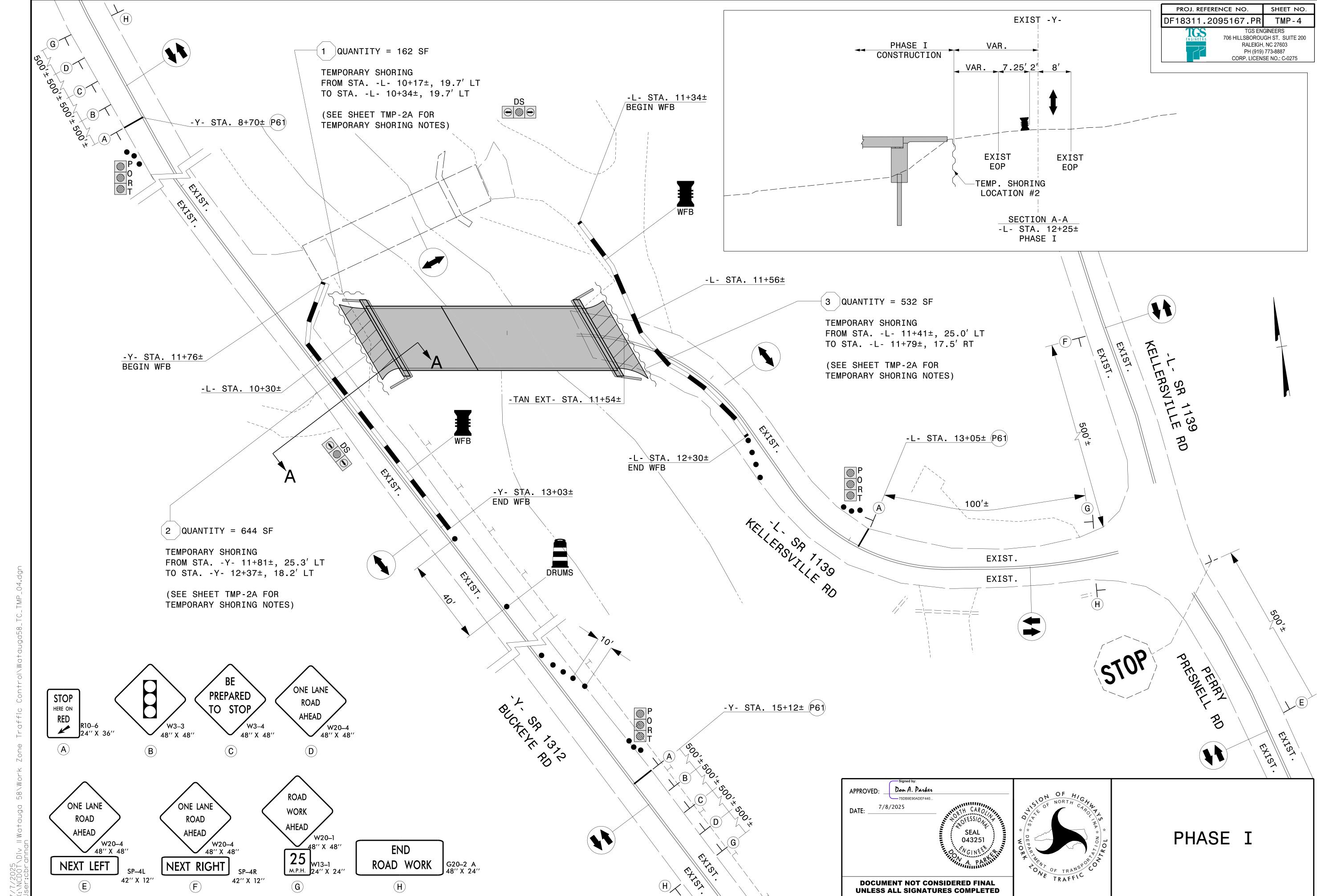
- STEP 1 -- USING FLAGGERS, REMOVE TEMPORARY PORTABLE SIGNAL, COVER SIGNAL WARNING SIGNS, REMOVE WATERFILLED BARRIER, AND OPEN PROJECT TO FINAL PATTERN.
- STEP 2 -- USING FLAGGERS, PLACE THE FINAL LAYER OF SURFACE COUSE THROUGHOUT THE PROJECT LIMITS.
- STEP 3 -- USING FLAGGERS, PLACE FINAL PAVEMENT MARKINGS AND PROPOSED SIGNING THROUGHOUT THE PROJECT LIMITS (SEE SIGNING AND PAVEMENT MARKING PLANS).
- STEP 4 -- REMOVE ALL TRAFFIC CONTROL DEVICES.



UNLESS ALL SIGNATURES COMPLETED



PHASING



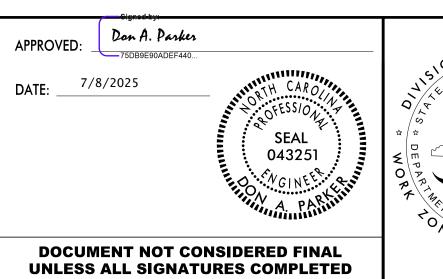
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

PORTABLE SIGNAL IN SAME LOCATION AS SHOWN ON TMP-4.

Docusign Envelope ID: 0AAF4D18-F0E7-48BB-AD94-092292BB603F PROJ. REFERENCE NO. SHEET NO. DF18311.2095167.PR TMP-6 TGS ENGINEERS 706 HILLSBOROUGH ST. SUITE 200 RALEIGH, NC 27603 PH (919) 773-8887 CORP. LICENSE NO.: C-0275 -Y- STA. 11+00± REMOVE EXISTING TEMPORARY BRIDGE CONTRACTOR SHALL MAINTAIN DRIVEWAY ACCESS AT ALL TIMES. -Y- STA. 10+00± / BEGIN WFB 码 -L- STA. 10+79± END WFB • (B) B 12' MIN. <u>-Y- STA. 11+93±</u> -L- STA. 12+50± <u>-TAN EXT- STA. 11+54±</u> TELLERS OF THE PO EXIST. R11-2 48" x 30" EXIST. == | TYPE III BARRICADE R11-2 48'' x 30'' BUCKEL AD TO TO

TYPE III BARRICADE B

ALL STATIONARY WORKZONE ADVANCE WARNING SIGNS FOR PROJECT LIMITS AND FOR TEMPORARY PORTABLE SIGNAL IN SAME LOCATION AS SHOWN ON TMP-4.



PHASE II

I.P.: DF18311.2095167.PK

RACT: DK00434

1262.01

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

PAVEMENT MARKING PLAN WATAUGA COUNTY

LOCATION: BRIDGE #640058 OVER BEECH CREEK ON SR 1139 (KELLERSVILLE RD) DF 18311.2095167.PR PMP - 1

APPROVED: Don A. Parker

75DB9E90ADEF440...

DATE: SEAL

043251 MGINEEL LOTER

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

INDEX

SHEET NO.

DESCRIPTION

PMP-1

PAVEMENT MARKING PLAN TITLE,
SCHEDULE SHEET, INDEX OF SHEETS,
LIST OF APPPLICABLE ROADWAY STANDARD
DRAWINGS, GENERAL NOTES, AND
FINAL PAVEMENT MARKING SCHEDULE

PMP-2

PAVEMENT MARKING DETAIL

ROADWAY STANDARD DRAWING

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2024 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO.		TITLE
1005 01	DAVEMENT MARKENSO	

1205.01	PAVEMENT MARKINGS - LINE TYPES AND OFFSETS
1205.02	PAVEMENT MARKINGS - TWO-LANE AND MULTILANE ROADWAYS
1205.04	PAVEMENT MARKINGS - INTERSECTIONS
1205.12	PAVEMENT MARKINGS - BRIDGES
1261.01	GUARDRAIL AND BARRIER DELINEATORS - INSTALLATION SPACING
1261.02	GUARDRAIL AND BARRIER DELINEATORS - TYPES AND MOUNTING

GUARDRAIL END DELINEATION

GENERAL NOTES

THE FOLLOWING GENERAL NOTES APPLY AT ALL TIMES FOR THE DURATION OF THE CONSTRUCTION PROJECT, EXCEPT WHEN OTHERWISE NOTED IN THE PLAN, OR DIRECTED BY THE ENGINEER.

A) INSTALL PAVEMENT MARKINGS AND PAVEMENT MARKERS ON THE FINAL SURFACE AS FOLLOWS:

ROAD NAME	MARKING	MARKER
-L- SR 1139 KELLERSVILLE RD	PAINT	NONE
-Y- SR 1312 BUCKEYE RD	PAINT	NONE

- B) PLACE TWO APPLICATIONS OF PAINT PAVEMENT MARKINGS ON THE FINAL WEARING SURFACE. PLACE THE SECOND APPLICATION OF PAINT UPON SUFFICIENT DRYING TIME OF THE FIRST.
- D) TIE PROPOSED PAVEMENT MARKING LINES TO EXISTING PAVEMENT MARKING LINES.
- E) REMOVE/REPLACE ANY CONFLICTING/DAMAGED PAVEMENT MARKINGS AND MARKERS.
- F) PASSING ZONES WILL BE DETERMINED IN THE FIELD AND MUST BE APPROVED BY THE ENGINEER.

FINAL PAVEMENT MARKING SCHEDULE

SYMBOL DESCRIPTION

PAVEMENT MARKINGS

PAINT (4'')

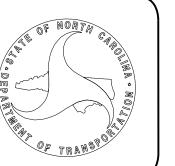
(4") WHITE EDGELINE

(4") 2 FT.-6 FT./SP WHITE MINISKIP

P13 (4") YELLOW DOUBLE CENTER

PLAN SUBMITTED TO: NCDOT

ROB WEISZ, P.E. DIVISION 11 BRIDGE PROGRAM MANAGER

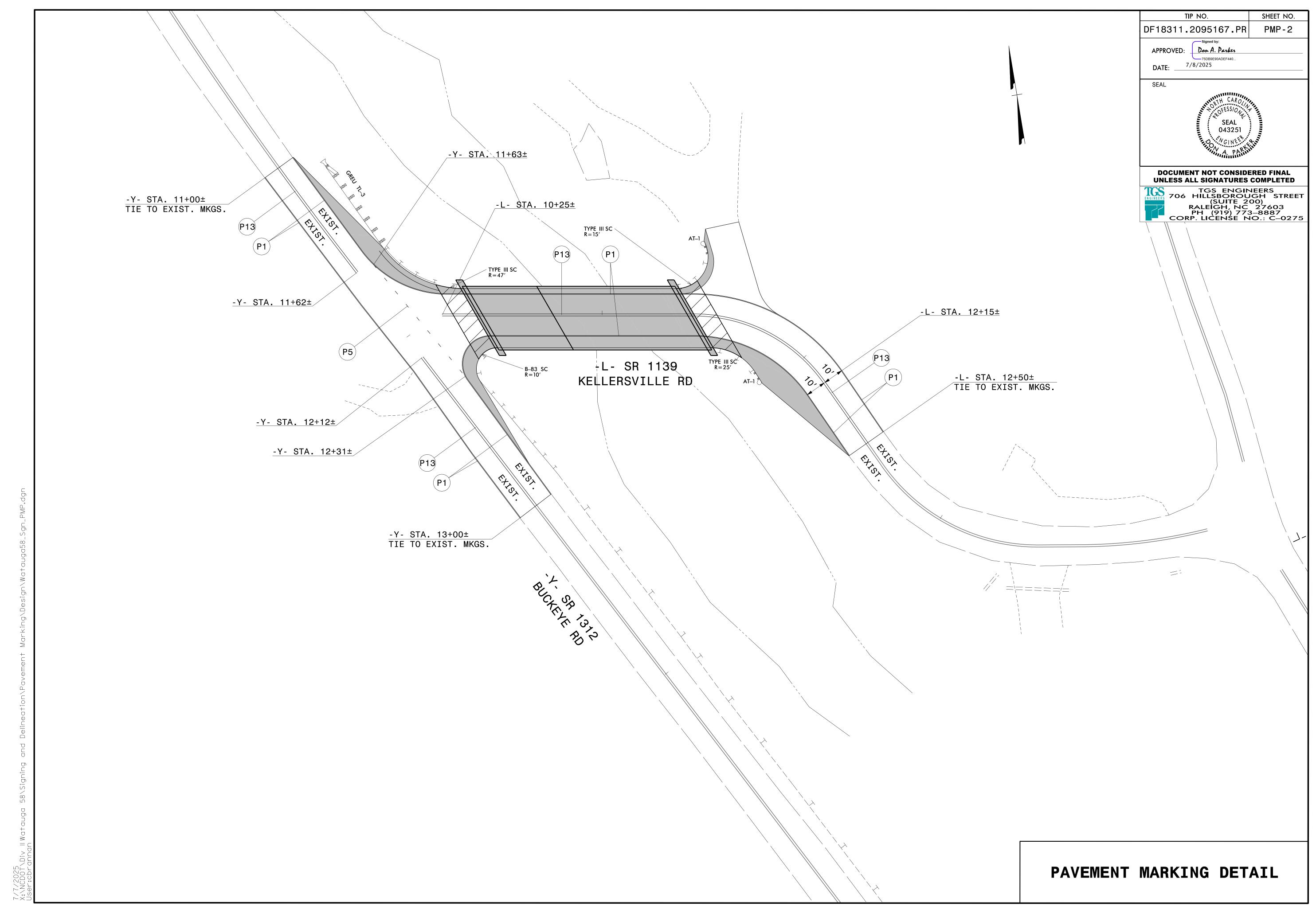


PLAN PREPARED BY: TGS ENGINEERS

DON A. PARKER, P.E.PROJECT ENGINEERCODA BRANNAN, E.I.DESIGN ENGINEER



TGS ENGINEERS
706 HILLSBOROUGH ST. SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275



Rominger

VICINITY MAP

NOT TO SCALE

STATE OF NORTH CAROLINA DIVISION OF HIGHWAYS

PLAN FOR PROPOSED HIGHWAY EROSION CONTROL

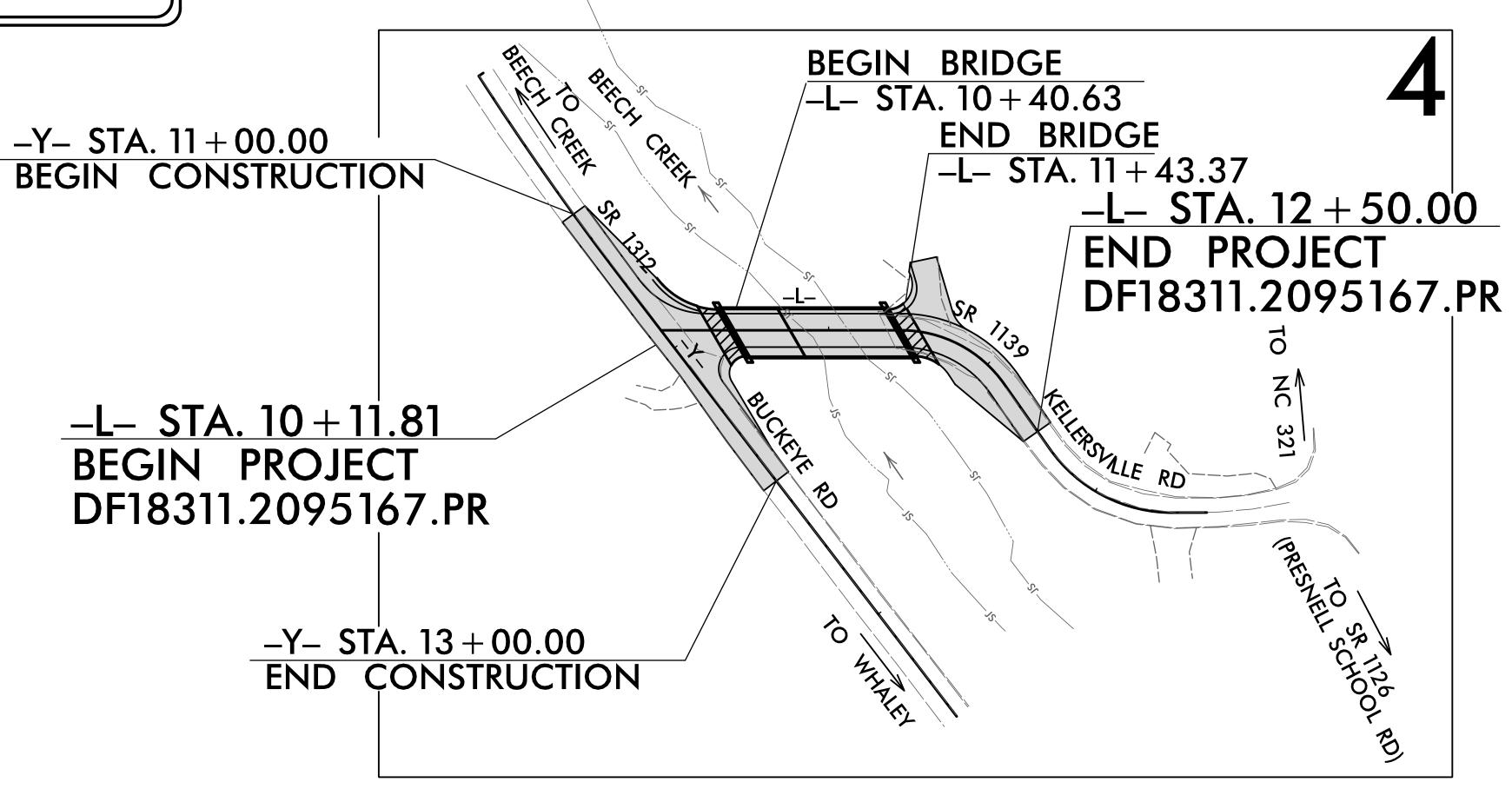
WATAUGA COUNTY

LOCATION: BRIDGE NO. 940058 OVER BEECH CREEK ON SR 1139 (KELLERSVILLE ROAD)

TYPE OF WORK: GRADING, DRAINAGE, PAVING, AND STRUCTURE



STATE	STATE	PROJECT REFERENCE NO.	NO.	SHEETS
N.C.	DF18	311.2095167.PR	EC-1	
STAT	E PROJ. NO.	F. A. PROJ. NO.	DESCRIPT	ION
DF18311.	2095167.PR	TBD	PE	
DF18311.	2095167.PR	TBD	R/W, U	TIL.
DF18311.	2095167.PR	TBD	CÓN	ST.



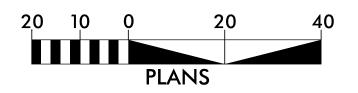
THIS PROJECT CONTAINS **EROSION CONTROL PLANS** FOR CLEARING AND **GRUBBING PHASE OF** CONSTRUCTION.

THIS PROJECT HAS **BEEN DESIGNED TO SENSITIVE WATERSHED** STANDARDS.

ENVIRONMENTALLY SENSITIVE AREA(S) EXIST ON THIS PROJECT

Refer To E. C. Special Provisions for Special Considerations.

GRAPHIC SCALE



THESE EROSION AND SEDIMENT CONTROL PLANS COMPLY WITH THE REGULATIONS SET FORTH BY THE NCG 010000 GENERAL STORMWATER CONSTRUCTION PERMIT ISSUED BY THE NORTH CAROLINA DEPARTMENT OF ENVIRONMENTAL QUALITY DIVISION OF ENERGY, MINERAL, AND LAND RESOURCES.



Prepared in the Office of:

TGS ENGINEERS 201 W. MARION ST-STE 200

SHELBY, NC 28150

Designed by:

Andrew H. Cochrane, PE

3015 LEVEL III CERTIFICATION NO.

Roadway Standard Drawings

The "Roadway Standard Drawings"- Roadway Design Unit - N. C. Department of Transportation - Raleigh, N. C., dated January 2024 and the latest revison thereto are applicable to this project and by reference hereby are considered a part of these plans.

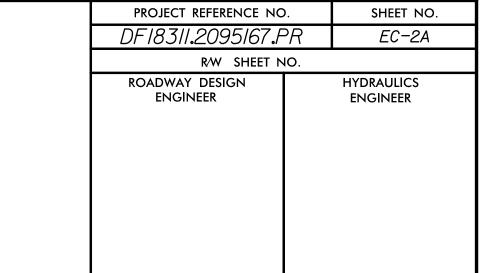
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

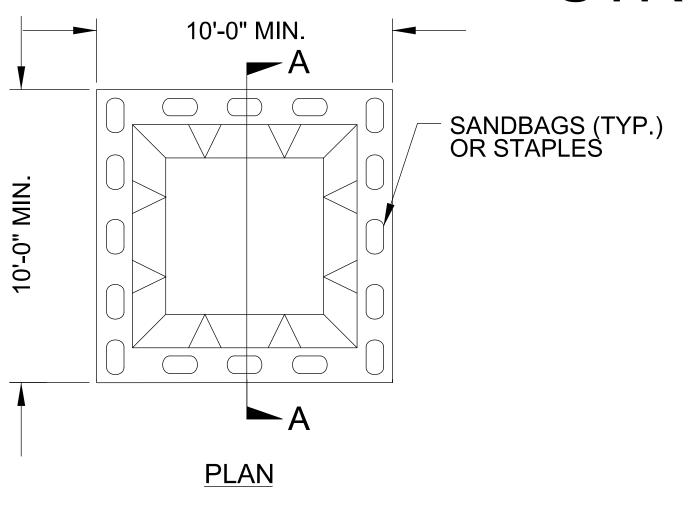
			SHEET NO.	
L	PROJECT REFERENCI	PROJECT REFERENCE NO.		
	DF18311.2095167.	F18311.2095167.PR		
	ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER	

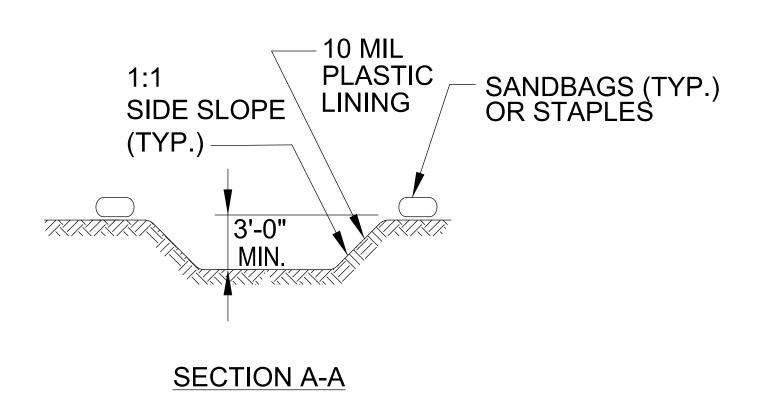
EROSION & SEDIMENT CONTROL LEGEND

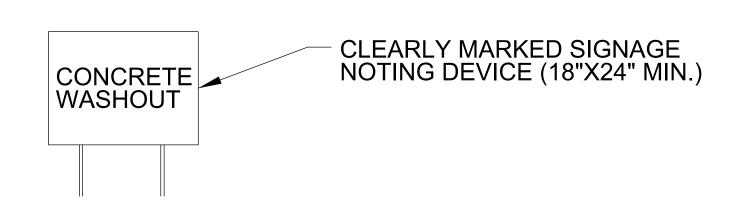
<u>Std. #</u>	<u>Description</u>	<u>Symbol</u>	Std. #	<u>Description</u>	Symbol
1605.01	Temporary Silt Fence	···· 	1633.01	Temporary Rock Silt Check Type A	
1606.01	Special Sediment Control Fence		1633.02	Temporary Rock Silt Check Type B	
1622.01	Temporary Berms and Slope Drains		1633.03	Temporary Rock Silt Check Type A with Excelsior Matting and Flocculant	
1630.02	Silt Basin Type B		1634.01	Temporary Rock Sediment Dam Type A	
1630.03	Temporary Silt Ditch	TSD	1634.02	Temporary Rock Sediment Dam Type B	
1630.04	Stilling Basin		1635.01	Rock Pipe Inlet Sediment Trap Type A	
1630.05	Temporary Diversion	TD	1635.02	Rock Pipe Inlet Sediment Trap Type B	B
1630.06	Special Stilling Basin		1636.01	Excelsior Wattle Check	
1630.07	Skimmer Basin		1636.01	Excelsior Wattle Check with Flocculant	
1630.08	Tiered Skimmer Basin		1636.01	Coir Fiber Wattle Check	
1630.09	Earthen Dam with Skimmer		1636.01	Coir Fiber Wattle Check with Flocculant	
	Infiltration Basin		1636.02	Silt Fence Excelsior Wattle Break	
1622.01	Rock Inlet Sediment Trap:	↑ 8		Silt Fence Coir Fiber Wattle Break	···
1632.01	Type A	₹ 0000000	1636.03	Excelsior Wattle Barrier	EW—EW—
1632.02	Type B		4000 00		
1632.03	Type C		1636.03	Coir Fiber Wattle Barrier	CFW—CFW—

ONSITE CONCRETE WASHOUT STRUCTURE WITH LINER









BELOW GRADE WASHOUT STRUCTURE

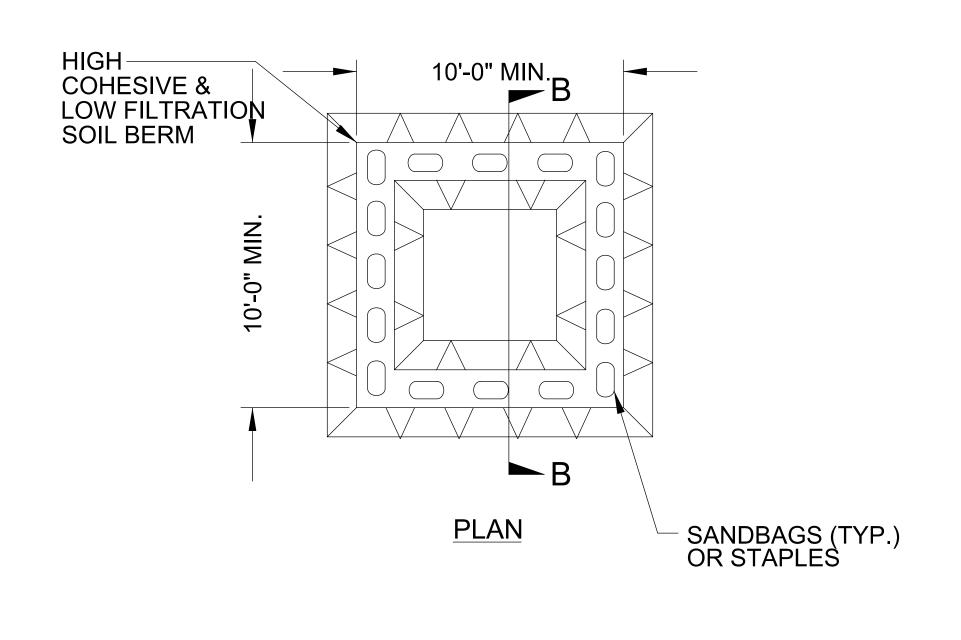
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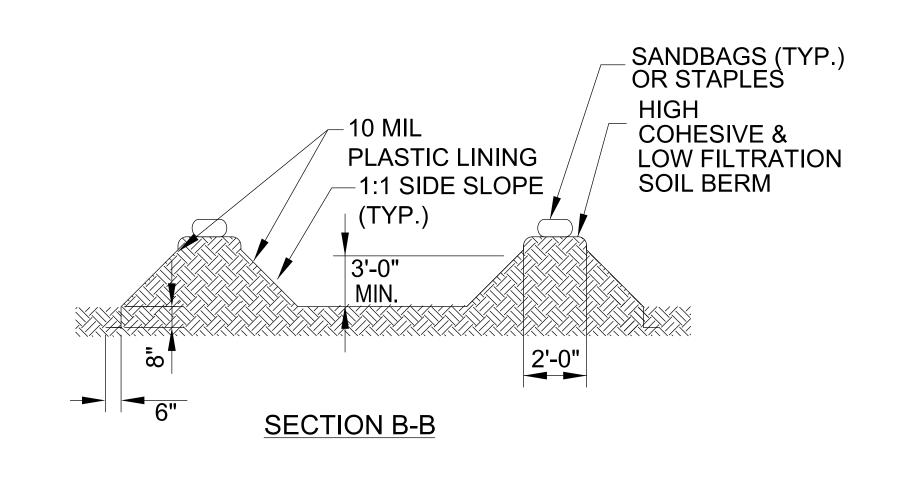
NOTES:

1. ACTUAL LOCATION DETERMINED IN FIELD

2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD.

3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE.





CLEARLY MARKED SIGNAGE NOTING DEVICE (18"X24" MIN.) WASHOUT

ABOVE GRADE WASHOUT STRUCTURE NOT TO SCALE

NOTES:

1. ACTUAL LOCATION DETERMINED IN FIELD

2. THE CONCRETE WASHOUT STRUCTURES SHALL BE MAINTAINED WHEN THE LIQUID AND/OR SOLID REACHES 75% OF THE STRUCTURES CAPACITY TO PROVIDE ADEQUATE HOLDING CAPACITY WITH A MINIMUM 12 INCHES OF FREEBOARD.

3.CONCRETE WASHOUT STRUCTURE NEEDS TO BE CLEARY MARKED WITH SIGNAGE NOTING DEVICE.

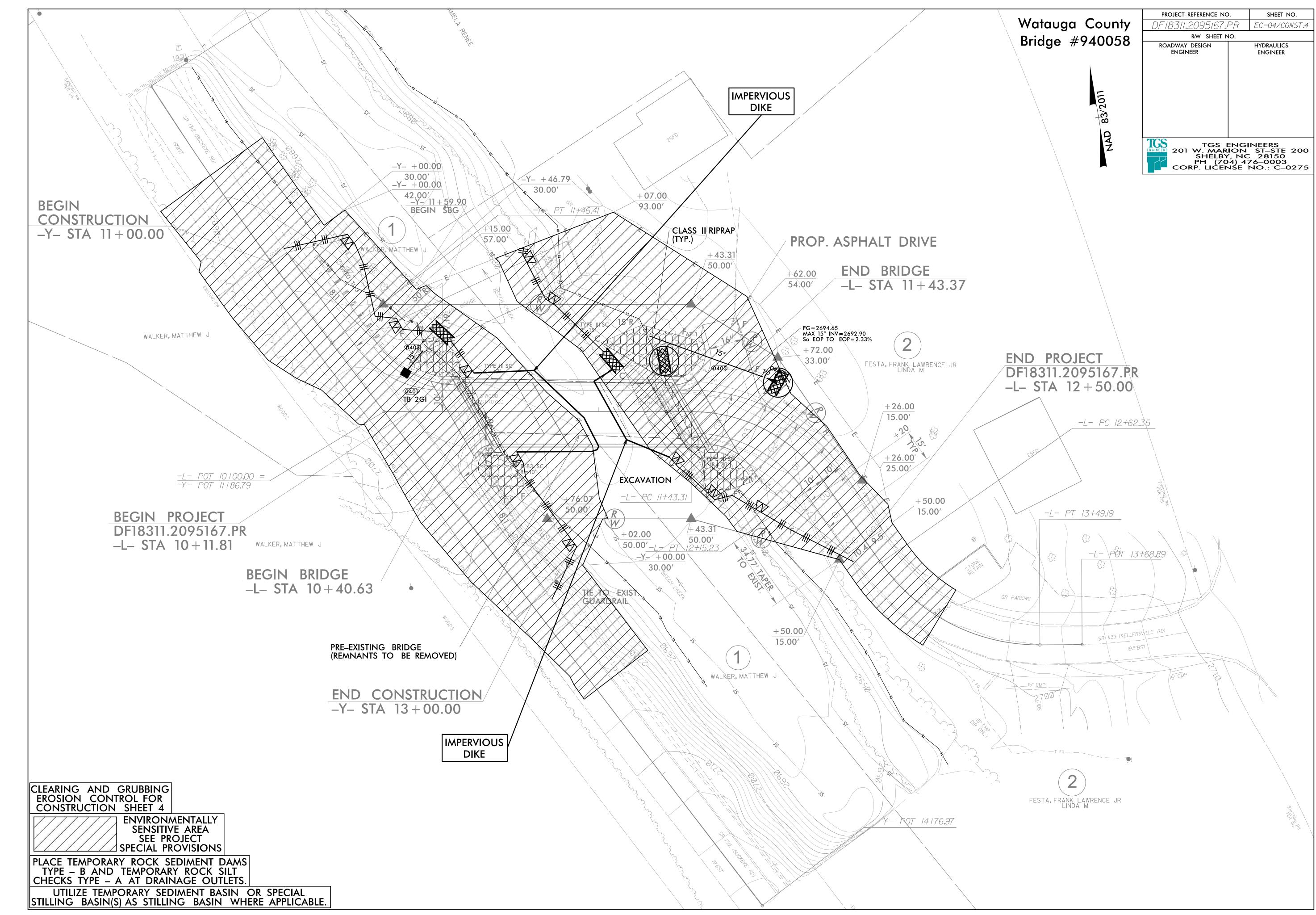
DIVISION OF HIGHWAYS STATE OF NORTH CAROLINA

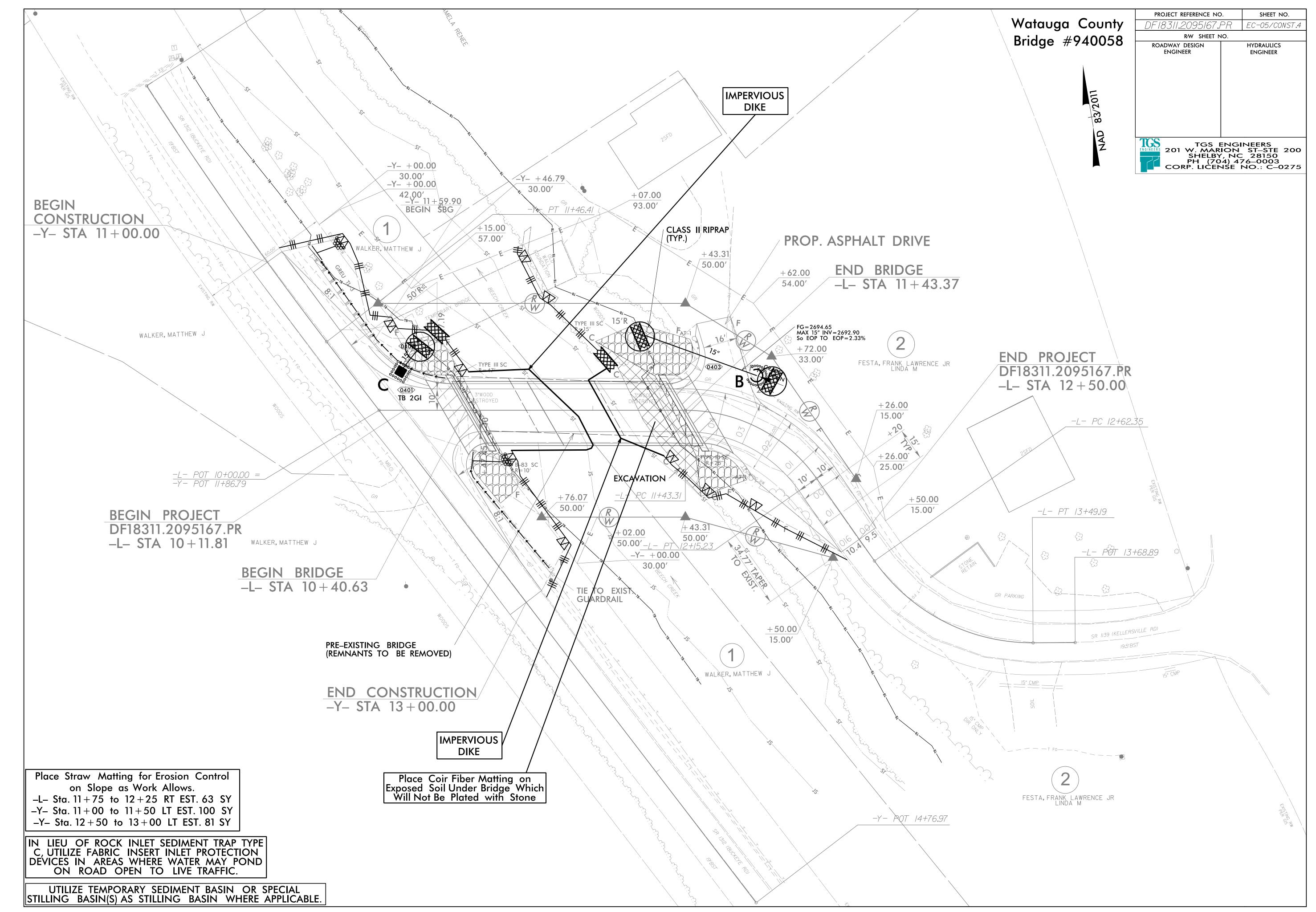
PROJECT REFERENCE NO	SHEET NO.	
DF18311 . 2095167 . PR		EC-3
ROADWAY DESIGN ENGINEER		HYDRAULICS ENGINEER

SOIL STABILIZATION TIMEFRAMES

SITE DESCRIPTION	STABILIZATION TIME	TIMEFRAME EXCEPTIONS
PERIMETER DIKES, SWALES, DITCHES AND SLOPES	7 DAYS	NONE
HIGH QUALITY WATER (HQW) ZONES	7 DAYS	NONE
SLOPES STEEPER THAN 3:1	7 DAYS	IF SLOPES ARE 10'OR LESS IN LENGTH AND ARE NOT STEEPER THAN 2:1, 14 DAYS ARE ALLOWED.
SLOPES 3:I TO 4:I		7 DAYS FOR SLOPES GREATER THAN 50'IN LENGTH WITH SLOPES STEEPER THAN 4:1.
SLUFES SILLO 4II	I4 DAYS	7 DAYS FOR PERIMETER DIKES, SWALES, DITCHES PERIMETER SLOPES, AND HQW ZONES
ALL OTHER AREAS WITH SLOPES FLATTER THAN 4:1	I4 DAYS	7 DAYS FOR PERIMETER DIKES, SWALES, DITCHES PERIMETER SLOPES, AND HQW ZONES

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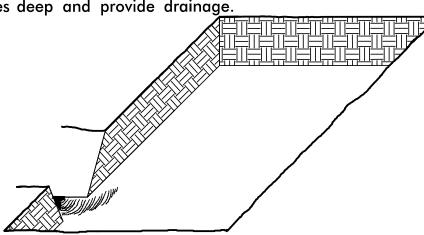


PLANTING DETAILS

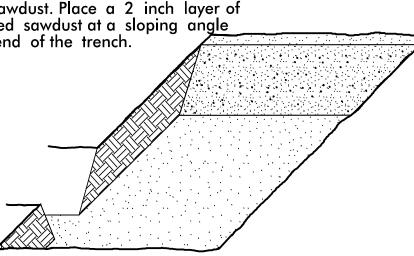
SEEDLING / LINER BAREROOT PLANTING DETAIL

HEALING IN

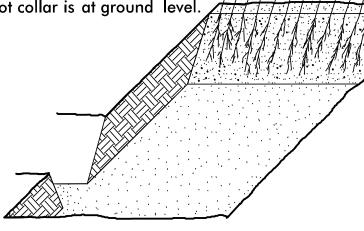
- Locate a healing—in site in a shady, well protected area.
- Excavate a flat bottom trench
 inches deep and provide drainage.

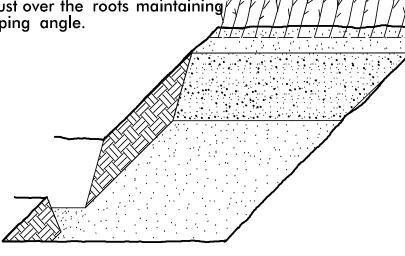


3. Backfill the trench with 2 inches well rotted sawdust. Place a 2 inch layer of well rotted sawdust at a sloping angle at one end of the trench.



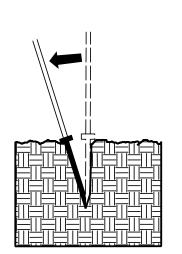
4. Place a single layer of plants against the sloping end so that the root collar is at ground level.



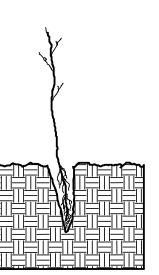


Repeat layers of plants and sawdust as necessary and water thoroughly.

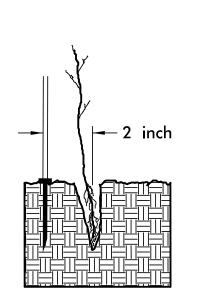
DIBBLE PLANTING METHOD USING THE KBC PLANTING BAR



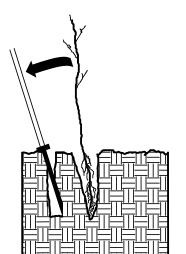
 Insert planting bar as shown and pull handle toward planter.



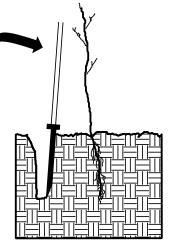
Remove planting bar and place seedling at correct depth.



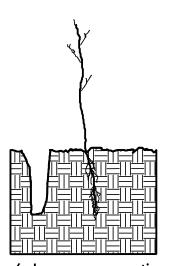
Insert planting bar
 inches toward planter
 from seedling.



 Pull handle of bar toward planter, firming soil at bottom.



Push handle forward firming soil at top.



Leave compaction hole open. Water thoroughly.

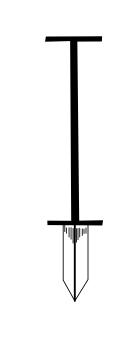
PLANTING NOTES:

PLANTING BAG
During planting, seedlings shall be kept in a moist canvas bag or similar container to prevent the root systems from drying.



KBC PLANTING BAR
Planting bar shall have a
blade with a triangular
cross section, and shall
be 12 inches long,
4 inches wide and
1 inch thick at center.

ROOT PRUNING
All seedlings shall be root pruned, if necessary, so that no roots extend more than 10 inches below the root collar.



	STATE	STATE	PROJECT REFERENCE NO.		SHEET NO.	TOTAL SHEETS
l	N.C.	DF183	?	RF-1		
ı	STATE	PROJ. NO.		DESCRIF	PTION	
l						

REFORESTATION

☐ TREE REFORESTATION SHALL BE PLANTED 6 FT. TO 10 FT. ON CENTER, RANDOM SPACING, AVERAGING 8 FT. ON CENTER, APPROXIMATELY 680 PLANTS PER ACRE.

REFORESTATION

MIXTURE, TYPE, SIZE, AND FURNISH SHALL CONFORM TO THE FOLLOWING:

25%	LIRIODENDRON TULIPIFERA	TULIP POPLAR	12 in – 18 in BR
25%	PLATANUS OCCIDENTALIS	SYCAMORE	12 in - 18 in BR
25%	BETULA NIGRA	RIVER BIRCH	12 in – 18 in BR
25%	NYSSA SYLVATICA	BLACK GUM	12 in – 18 in BR

REFORESTATION DETAIL SHEET

N.C.D.O.T. – ROADSIDE ENVIRONMENTAL UNIT

.P.: DF18311.2095167.PR

TRACT: DK00434

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

SIGNING PLAN WATAUGA COUNTY

LOCATION: BRIDGE #940058 OVER BEECH CREEK
ON SR 1139 (KELLERSVILLE RD)

TIP NO. SHEET NO.

DF18311.2095167.PR SIGN-1

APPROVED: Dan A. Parker

7/8/2025

DATE: _



DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

ROADWAY STANDARD DRAWING

THE FOLLOWING ROADWAY STANDARDS AS APPEAR IN "ROADWAY STANDARD DRAWINGS" - PROJECT SERVICES UNIT - N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2024 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

STD. NO. TITLE

904.10 ORIENTATION OF GROUND MOUNTED SIGNS

904.50 MOUNTING OF TYPE 'D', 'E' AND 'F' SIGNS ON 'U' CHANNEL POSTS

		SUMMARY OF QUANTITIES	1		١
ITEM I	NO.	ITEM DESCRIPTION	QUANTITY	UNIT	
DESC. NO.	SECT. NO.				
4072000000	903	SUPPORTS, 3 LB STEEL U-CHANNEL	77	L.F.	
4102000000	904	SIGN ERECTION, TYPE E	3	EA.	
4155000000	907	DISPOSAL OF SIGN SYSTEM, U-CHANNEL	2	EA.	

GENERAL NOTES

- . SIGNS FURNISHED BY STATE
- . CONFIRM IN WRITING AT LEAST 4 MONTHS IN ADVANCE, THE ACTUAL DATE THE DEPARTMENT FURNISHED SIGNS WILL BE REQUIRED.
- . IF REMOVAL OR RELOCATION OF SIGNS ON PRIVATE STREET (NON-STATE MAINTAINED) IS REQUIRED DUE TO CONSTRUCTION, THE CONTRACTOR SHALL INFORM THE ENGINEER. THE WORK WILL BE COMPLETED BY OTHERS.
- . WHEN NOT STATIONED OR DIMENSIONED ON PLANS, ALL 'E' AND 'F' SIGNS SHALL BE FIELD LOCATED BY THE ENGINEER
- . ALL EXISTING SIGNS ON "U" CHANNEL POST WITHIN THE PROJECT LIMITS SHALL BE REMOVED AND DISPOSED OF UNLESS OTHERWISE NOTED ON PLANS.
- . THE BACKGROUND FOR TYPE E & F SIGNS SHALL BE TYPE C REFLECTIVE SHEETING.
- . SEE ROADWAY PLANS FOR GUARD/GUIDE RAIL DETAILS.

INDEX

SHEET NO. DESCRIPTION

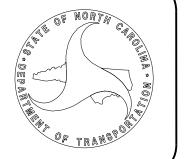
SIGN-1 TITLE SHEET

SIGN-2 TYPE E SIGNS

SIGN DETAIL SHEETS

PLAN SUBMITTED TO: NCDOT

ROB WEISZ, P.E. DIVISION 11 BRIDGE PROGRAM MANAGER



PLAN PREPARED BY: TGS ENGINEERS

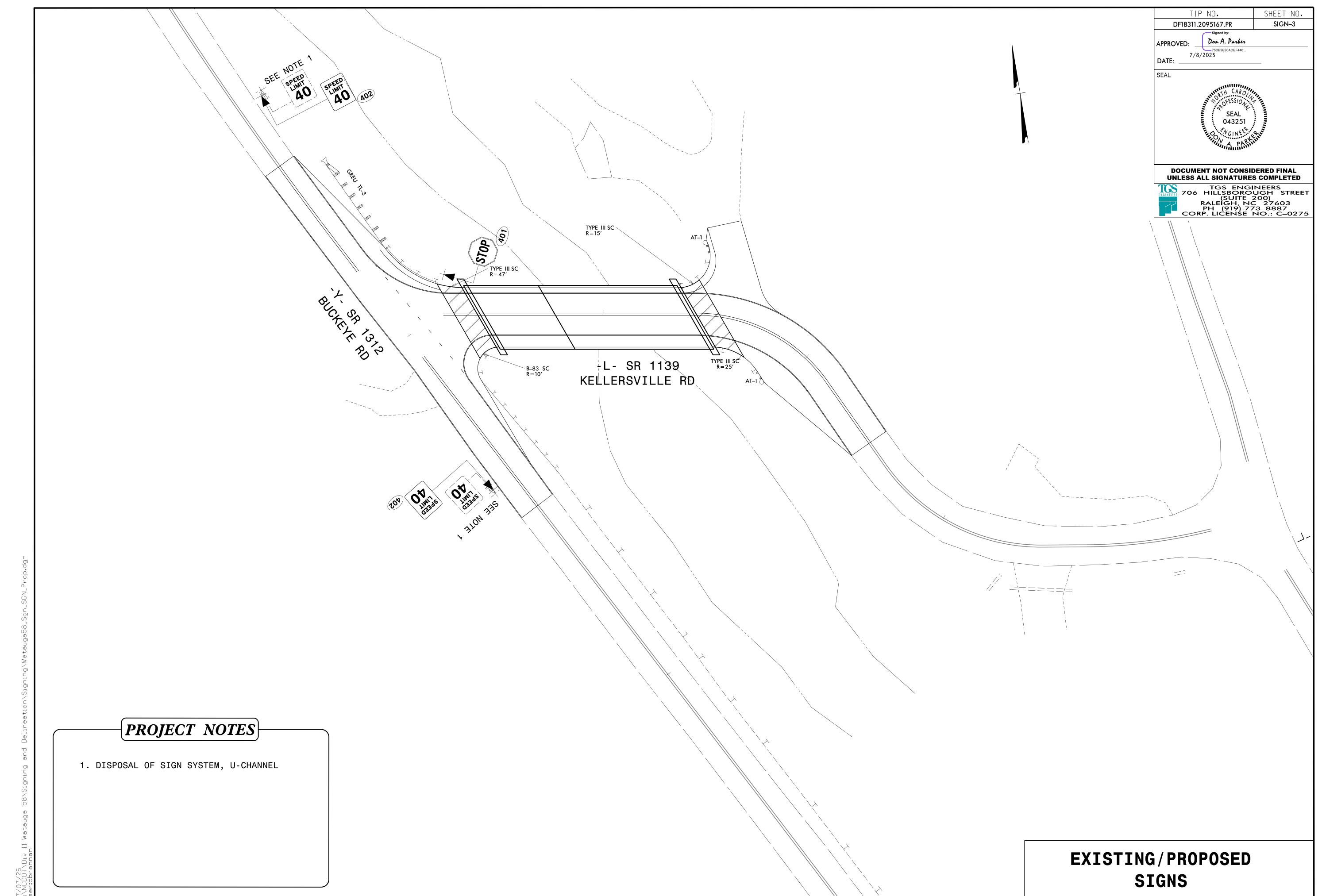
DON A. PARKER, P.E. PROJECT ENGINEER
CODA BRANNAN, E.I. DESIGN ENGINEER



TGS ENGINEERS
706 HILLSBOROUGH ST. SUITE 200
RALEIGH, NC 27603
PH (919) 773-8887
CORP. LICENSE NO.: C-0275

401 QUANTITY REQ'D _1_		TIP NO. SHEET NO. DF18311.2095167.PR SIGN-2 Signed by: Don A. Parker
STOP 30 X 30 R1-1		DATE: SEAL SEAL SEAL O43251 OKANGINEE OKANGINEE
ONE "U" POST PER SIGN		DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 706 HILLSBOROUGH STREET (SUITE 200) RALEIGH, NC 27603 PH (919) 773–8887 CORP. LICENSE NO.: C-0275
402 QUANTITY REQ'D _2_		
SPEED LIMIT 24 X 30 R2-1		
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PROJECT REFERENCE NO. SHEET NO.

DF18311.2095167.PR X-1

R/W SHEET N

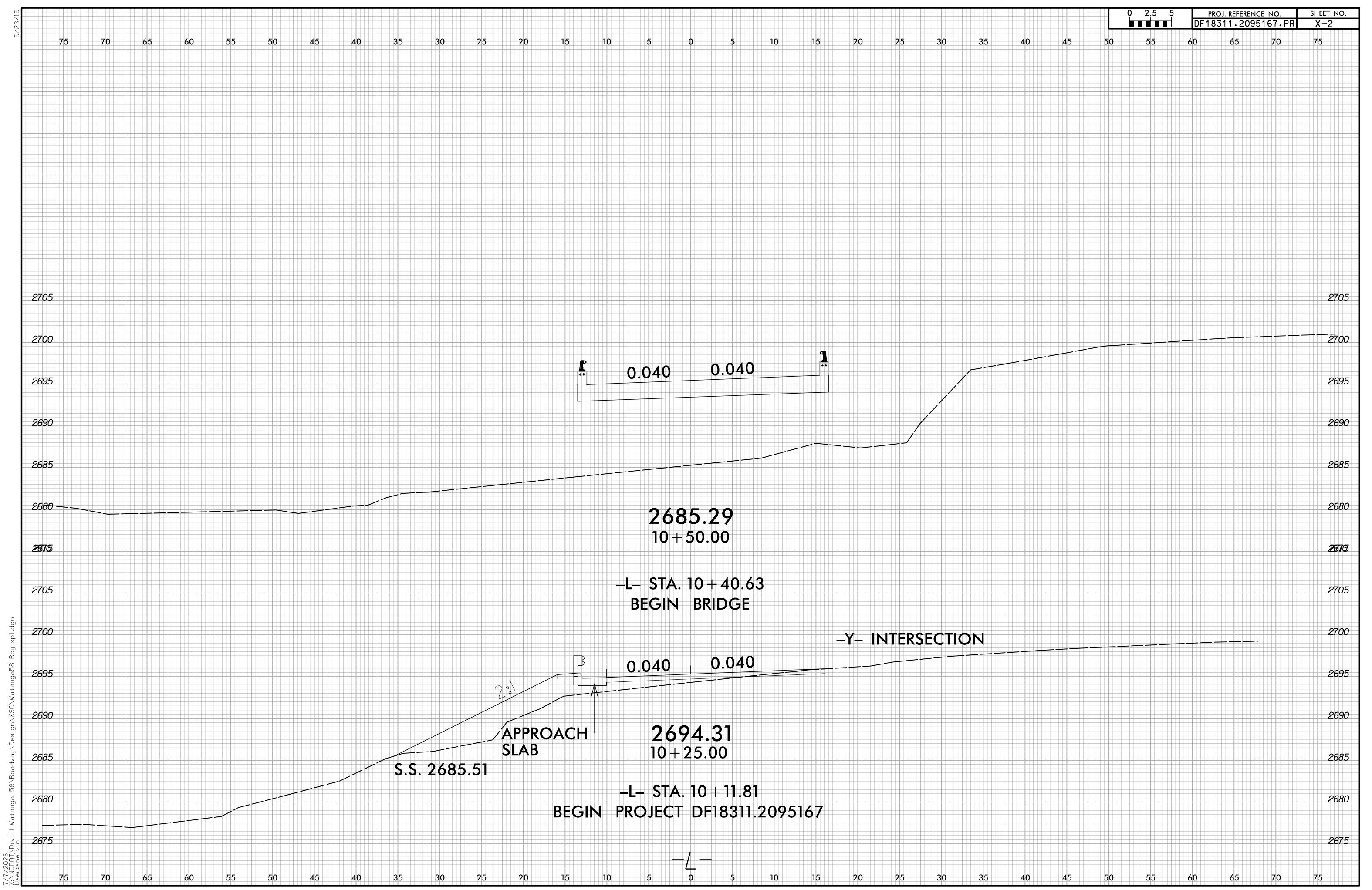
DF18311.2095167.PR CROSS-SECTION INDEX

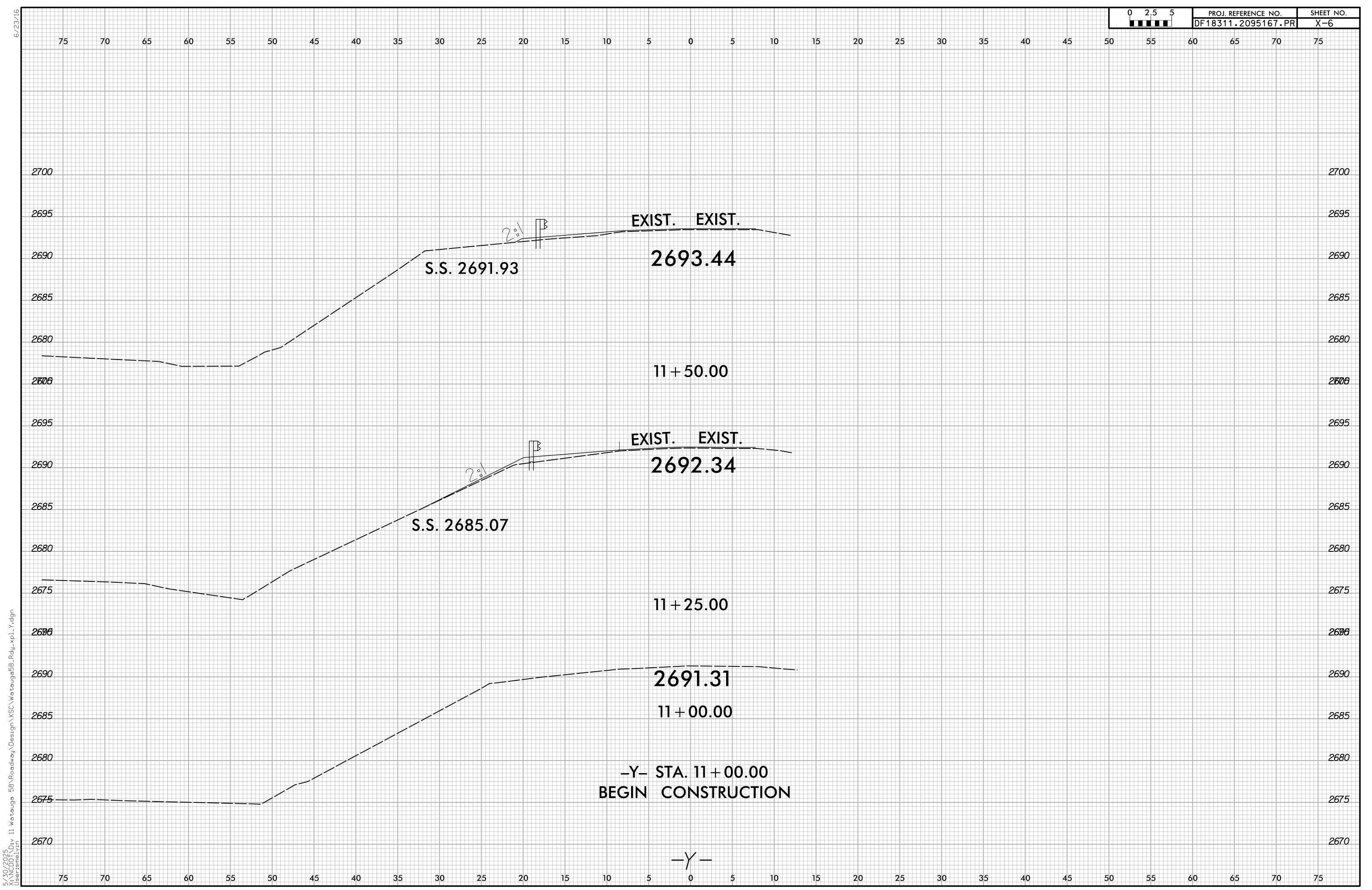
XS-INDEX X-1
XS-SUMMARY X-1A
-L- X-2 THRU X-5
-Y- X-6 THRU X-8

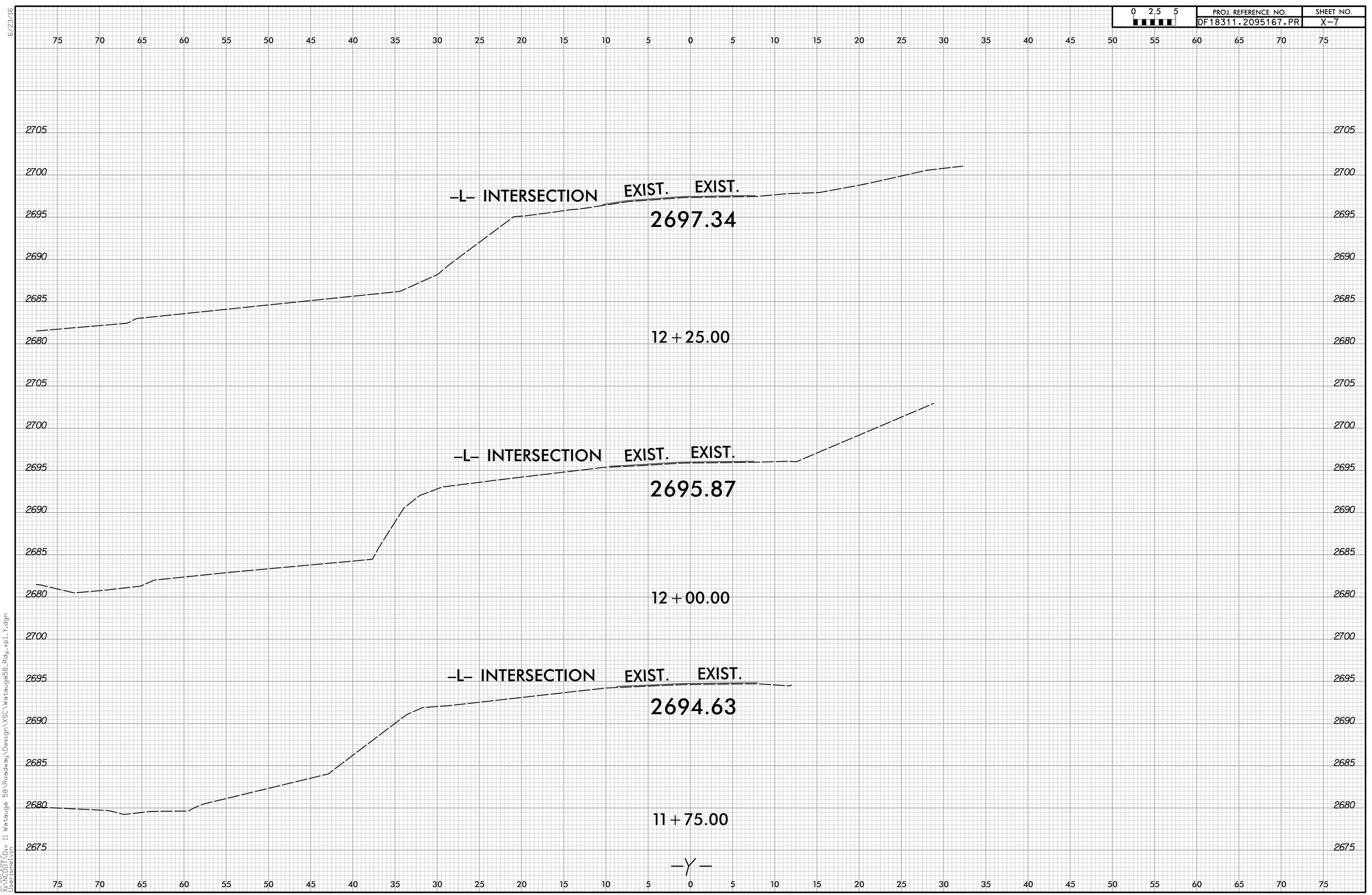
STATE OF NORTH CAROLINA **DIVISION OF HIGHWAYS**

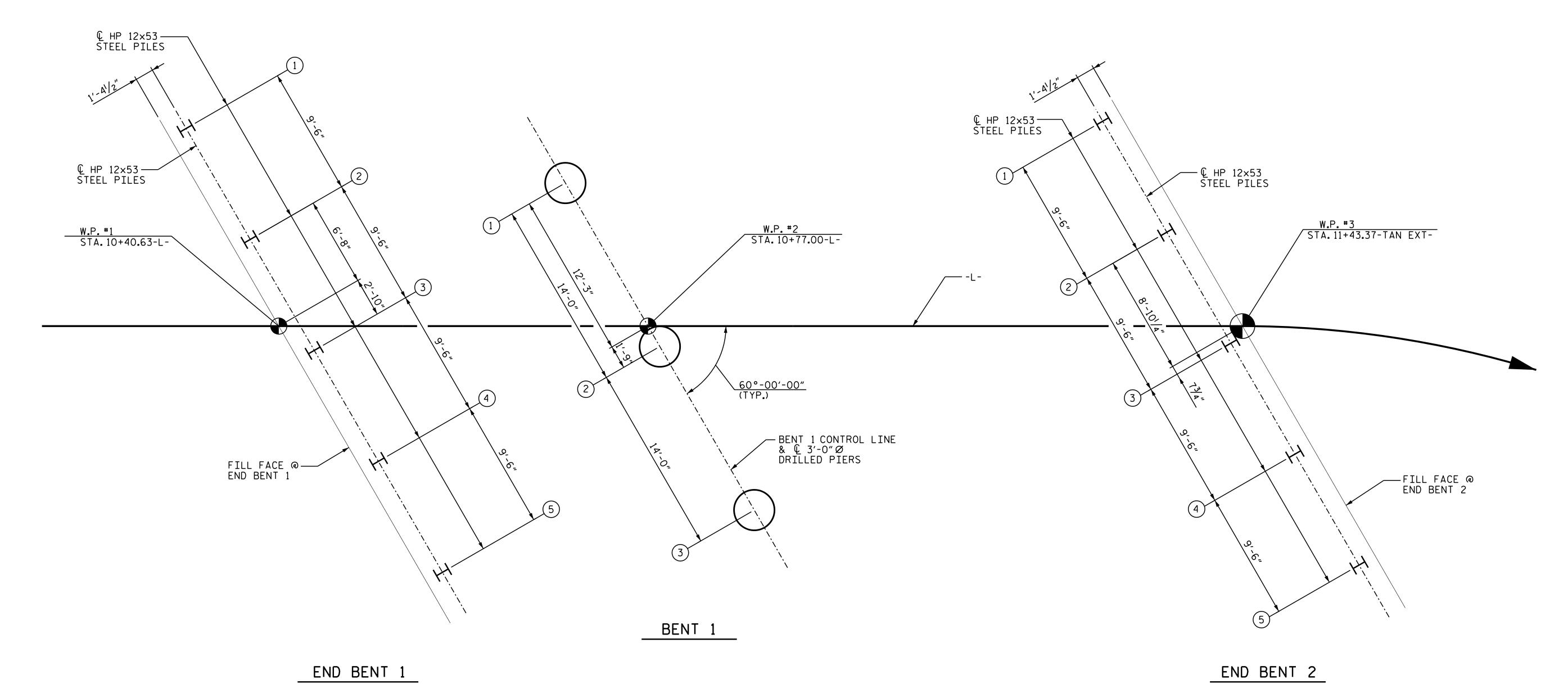
PROJ. REFERENCE NO. SHEET NO. DF18311.2095167.PR

NOTE: EMBANKM	ENT COLUMN DOES	S NOT INCLUDE I	BACKFILL FOR UNDERCUT	ROSS-SECTION SUMMA	RY	
Station	Uncl. Exc.	Embt				
L	(cu. yd.)	(cu. yd.)				
10+11.81	0	0				
10+25.00	1	16				
10+40.63	2	38				
Station	Unal Eva	Embt				
Station	Uncl. Exc.	Embt				
L	(cu. yd.)	(cu. yd.)				
11+43.37	0	0				
11+50.00	1	6				
11+75.00	1	29				
12+00.00	2	33				
12+25.00	6	20				
12+50.00	4	5				
Station	Uncl. Exc.	Embt				
Y	(au vd)	(au)(d)	Approximate quantities only. Unclassified e			
•	(cu. yd.)	(cu. yd.)	excavation, fine grading, clearing and grubb			
11+00.00 11+25.00	0	3	pavement will be paid for at the lump sum p	rice for "Grading".		
11+50.00	0	5				
11+75.00	0	3				
Station	Uncl. Exc.	Embt				
Υ	(cu. yd.)	(cu. yd.)				
12+25.00	0	0				
12+50.00	0	3				
12+75.00 13+00.00	0	10				
13+00.00	U	0				









NOTES:

FOR PILES, SEE SECTION 450 OF THE STANDARD SPECIFICATIONS.

FILL HOLES FOR PILE EXCAVATION AT END BENT 1 AND END BENT 2 WITH CONCRETE.

FOR DRILLED PIERS, SEE SECTION 411 OF THE STANDARD SPECIFICATIONS.

IF WEATHERED ROCK IS ENCOUNTERED AFTER PILE EXCAVATION AT END BENT 2, DRIVE PILES TO THE REQUIRED DRIVING RESISTANCE BEFORE BACKFILLING HOLES WITH CONCRETE.

PERMANENT CASINGS BEFORE EXCAVATING OR DISTURBING ANY MATERIAL BELOW ELEVATION 2,676 FT.

DRILLED PIERS AT BENT 1 REQUIRE PILOT BORINGS THAT WILL BE USED TO DETERMINE THE REQUIRED TIP NO HIGHER THAN ELEVATION. EACH BORING AT BENT 1 (PIER 1) SHALL BE ADVANCED TO AN ELEVATION OF 2,647 FEET AND BENT 1 (PIERS 2-3) TO AN ELEVATION OF 2,635 FEET. THE ENGINEER WILL REVIEW THE RESULTS INCLUDING ROCK CORES TO DETERMINE THE TIP ELEVATION FOR EACH DRILLED PIER. SEE GEOTECHNICAL SPECIAL PROVISION FOR PILOT BORINGS.

FOUNDATION LAYOUT PLAN

ALL END BENT PILES ARE HP 12×53 STEEL PILES.DIMENSIONS LOCATING PILES ARE SHOWN TO THE CENTERLINE OF PILES.ORIENT PILES AS SHOWN.

PROJECT NO.DF18311.2095167.PR

WATAUGA _ COUNTY

10+92.00-L-STATION:

SHEET 2 OF 5



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

GENERAL DRAWING

FOR BRIDGE OVER BEECH CREEK ON SR 1139 BETWEEN SR 1312 AND SR 1125

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS
201 W. MARION ST STE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C–0275

SHEET NO. REVISIONS S-2 NO. BY: DATE: BY: DATE: TOTAL SHEETS 26

INSTALL PERMANENT STEEL CASINGS AT BENT 1 BY VIBRATING, SCREWING OR DRIVING

DATE: 5/25 DRAWN BY : DATE: 6/25 CHECKED BY :

SUMMARY OF PILE INFORMATION/INSTALLATION

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)

	Number					Driven Piles			Predrilling For Piles *			Drilled-in-Piles		
End Bent/ Bent No. Pile(s) *-* (e.g., "Bent 1, Piles 1-5")	Number of Piles per Line	Factored Resistance per Pile KIPS	Pile Cut-Off (Top of Pile) Elevation FT	Estimated Pile Length per Pile FT	Scour Critical Elevation FT	Min. Pile Tip (Tip No Higher Than) Elev. FT	Required Driving Resistance (RDR)** per Pile KIPS	Pile Redrives Quantity EACH	Predrilling Length per Pile LIN FT	Predrilling Elevation (Elev Not To Predrill Below) FT	Maximum Predrilling Dia INCHES	Pile Excavation (Bottom of Hole) Elev FT	Pile Exc Not In Soil per Pile LIN FT	Pile Exc In Soil per Pile LIN FT
End Bent 1, Piles 1-3	3	132		15								2676	4.40	4.60
End Bent 1, Piles 4-5	2	132	SEE	15								2677	5.80	2.20
End Bent 2, Piles 1-3	3	188	END BENT	25			314					2669	5.20	11.50
End Bent 2, Piles 4-5	2	188	SHEETS	25				1				2665	11.10	9.50
TOTAL QUANTITY:													62.60	71.70

* Predrilling for Piles is required for end bents/bents with a predrilling length and at the Contractor's option for end bents/bents with predrilling information but no predrilling length.

***RDR = Factored Resistance + Factored Drag Load + Factored Dead Load + Nominal Drag Load Resistance + Nominal Resistance From Scourable Material

Dynamic Resistance Factor

PILE DESIGN INFORMATION

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)

End Bent/ Bent No. Pile(s) #-# (e.g., "Bent 1, Piles 1-5")	Factored Axial Load per Pile KIPS	Factored Drag Load per Pile KIPS	Factored Dead Load * per Pile KIPS	Dynamic Resistance Factor	Nominal Drag Resistance per Pile KIPS	Nominal Scour Resistance per Pile KIPS
End Bent 1, Piles 1-5	132					
End Bent 2, Piles 1-3	188			0.6		
End Bent 2, Piles 4-5	188					

* Factored Dead Load is factored weight of pile above the groundline.

SUMMARY OF DRILLED PIER INFORMATION/INSTALLATION

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)

End Bent/ Bent No. Piers) *-* (e.g., "Bent 1, Piers 1-3")	Number of Piles per Line	Factored Resistance per Pier KIPS	Required Drilled Pier Tip Elevation FT	Required Tip Resistance Per Pier KSF	Elevation	Minimum Drilled Pier Penetration Into Rock per Pier LIN FT	Drilled Pier Length * per Pier LIN FT	Drilled Pier Length Not In Soil* per Pier LIN FT	Drilled Pier Length In Soil * per Pier LIN FT	Permanent Steel Casing Required? YES or MAYBE	Permanent Steel Casing Tip Elevation (Elev Not To Extend Casing Below) FT	Permanent Steel Casing Length** per Pier LIN FT
Bent 1, Pier 1	1	680	2657.00	10	2674.00	13.00		16.00	12.00	YES	2670.00	15.00
Bent 1, Piers 2-3	2	680	2645.00	10	2674.00	31.00		31.00	9.00	YES	2675.00	10.00
TOTAL QUANTITY:								78.00	30.00			35.00

- * Drilled Pier Length, Drilled Pier Length Not in Soil and Drilled Pier Length in Soil represent estimated drilled pier quantities and are measured and paid for as either "__ Dia. Drilled Piers" or "__ Dia. Drilled Piers Not in Soil" and "__ Dia. Drilled Piers in Soil" in accordance with Article 411-7 of the NCDOT Standard Specifications. For bents with a not in soil pay item, drilled piers through air or water will be paid at the contract unit price for "__Dia. Drilled Piers in Soil."
- ** Permanent Steel Casing Length equals the difference between the ground line or top of drilled pier elevation, whichever is higher, and the permanent casing tip elevation and is measured and paid for as "Permanent Steel Casing for "_Dia.Drilled Pier" in accordance with Article 411-7 of the NCDOT Standard Specifications.

NOTES:

- 1. The Pile Foundation Tables are based on the bridge substructure design and foundation recommendations sealed by a North Carolina Professional Engineer (Michael J. Walko, #026917) on 06-06-2025.
- 2. Total Pile Driving Equipment Setup quantity (not shown in Pile Foundation Tables) equals the number of driven piles, ie., the number of piles with a Required Driving Resistance.
- 3. The Engineer may adjust the quantity for DPT Testing, Pipe Pile Plates, Permanent Steel Casing, SPTs, TIPs, CSL Testing, SID Inspections and PITs when necessary.

DRAWN BY: ZCS DATE: 5/25 CHECKED BY: MGC DATE: 6/25

SUMMARY OF PILE ACCESSORIES

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)

		Steel Pile Points						
End Bent/ Bent No. Pile(s) *-* (e.g., "Bent 1, Piles 1-5")	Pipe Pile Plates EACH	Pipe Pile Cutting Shoes EACH	Pipe Pile Conical Points EACH	H-Pile Points EACH				
End Bent 2, Piles 1-3				3				
TOTAL QUANTITY:				3				

SUMMARY OF DRILLED PIER TESTING

(BLANK ENTRIES INDICATE ITEM IS NOT APPLICABLE TO STRUCTURE)

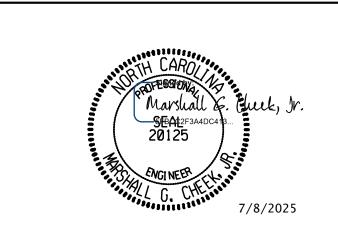
End Bent/ Bent No. PiER(s) #-# (e.g., "Bent 1, Piers 1-3")	Standard Penetration Test (SPT) Required? EACH	Crosshole Sonic Logging (CSL) EACH	Thermal Integrity Profiler (TIP) EACH	Shaft Inspection Device (SID) Required? EACH	Pile Integrity Test (PIT) Required? EACH
Bent 1, Piers 2-3		1			
TOTAL QUANTITY:		1			

PROJECT NO. DF18311.2095167.PR

WATAUGA COUNTY

STATION: 10+92.00-L-

SHEET 3 OF 5



DEPARTMENT OF TRANSPORTATION
RALEIGH

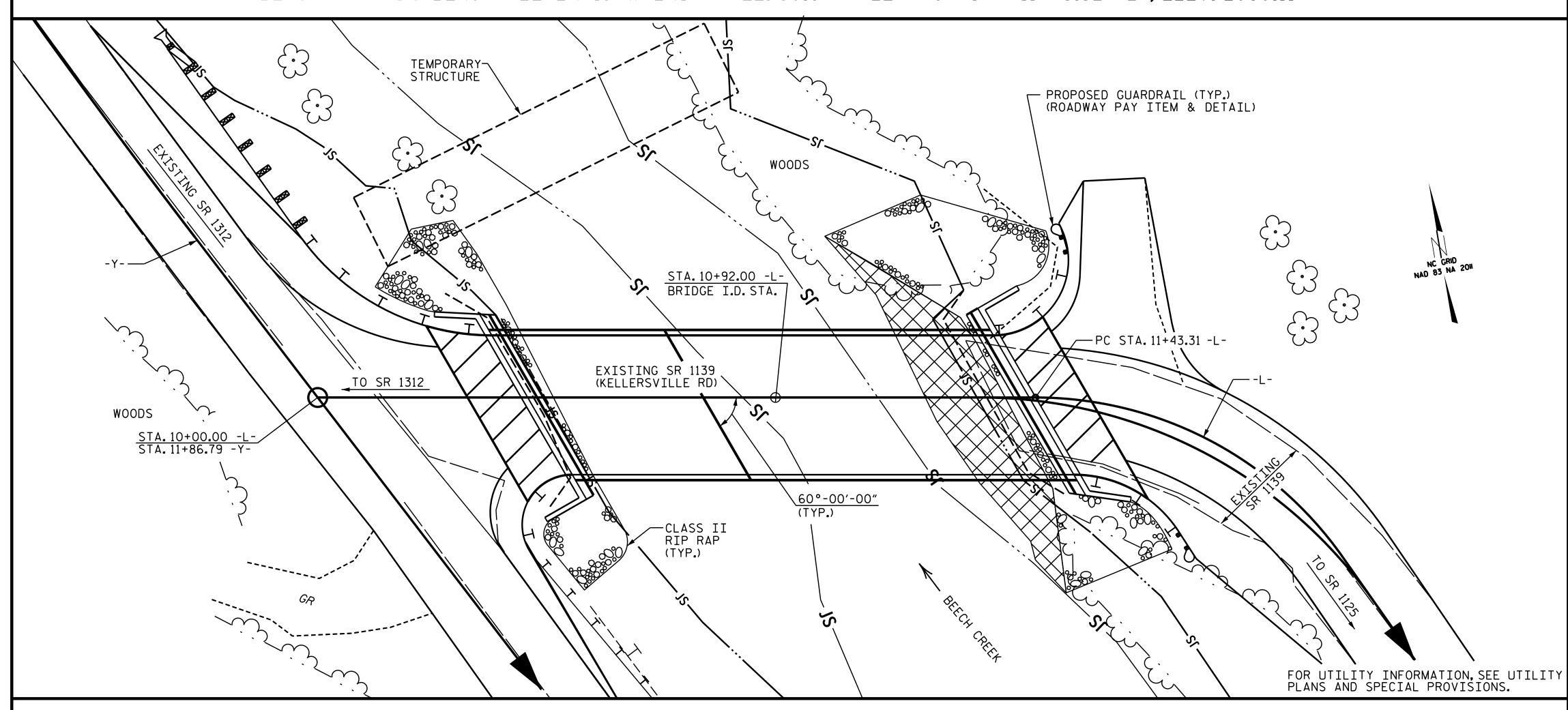
GENERAL DRAWING

FOUNDATION TABLES

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
201 W. MARION ST STE 200 SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C-0275 2 4 4 56

BENCHMARK #1: BENCH TIE IN 15" WALNUT TREE: 36.9 FT LEFT OF STA 11+73.32 -L-; ELEV. 2696.11



LOCATION SKETCH

NOTES

ASSUMED LIVE LOAD = HL-93 OR ALTERNATE LOADING.

FOR OTHER DESIGN DATA AND GENERAL NOTES, SEE STANDARD NOTES SHEET.

FOR EROSION CONTROL MEASURES, SEE EROSION CONTROL PLANS.

THIS BRIDGE HAS BEEN DESIGNED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

THE 3-SPAN BRIDGE (1 @ 25-10",1 @ 40'-0",1 @ 24'-10") CONSISTING OF A STEEL PLANK FLOOR ON STEEL I-BEAMS, WAS WASHED OUT AS A RESULT OF HURRICANE HELENE. A TEMPORARY STRUCTURE CONSISTING OF A RAIL CAR WAS PLACED BY NCDOT FORCES DOWNSTREAM OF THE PROPOSED STRUCTURE. THIS TEMPORARY STRUCTURE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE INTEGRITY OF THE BRIDGE DETERIORATE, THIS LOAD LIMIT MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT. FOR REMOVAL OF EXISTING STRUCTURE, SEE SPECIAL PROVISIONS.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED IN A MANNER THAT PREVENTS DEBRIS FROM FALLING INTO WATER. THE CONTRACTOR SHALL SUBMIT DEMOLITION PLANS FOR REVIEW AND REMOVE THE BRIDGE IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

THE MATERIAL SHOWN IN THE CROSS-HATCHED AREA SHALL BE EXCAVATED FOR THE DISTANCE OF 35 FT.LT. AND 40 FT.RT. OF -L- @ EB2 AS DIRECTED BY THE ENGINEER. THIS WORK WILL BE PAID FOR AT THE CONTRACT LUMP SUM PRICE FOR UNCLASSIFIED STRUCTURE EXCAVATION, SEE SECTION 412 OF THE STANDARD SPECIFICATIONS.

THIS STRUCTURE HAS BEEN DESIGNED IN ACCORDANCE WITH HEC 18, "EVALUATING SCOUR AT BRIDGES".

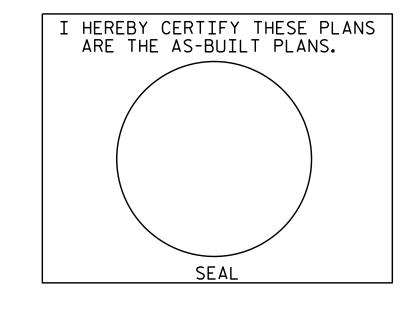
- FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.
- THIS BRIDGE IS LOCATED IN SEISMIC ZONE 1.
- FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.
- FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.
- FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.
- FOR ASBESTOS ASSESSMENT, SEE SPECIAL PROVISIONS.

THE CONTRACTOR WILL BE REQUIRED TO CONSTRUCT, MAINTAIN AND AFTERWARDS REMOVE A TEMPORARY ACCESS AT STATION 10+92.00-L- FOR USE DURING CONSTRUCTION OF THE PROPOSED STRUCTURE. FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS, SEE SPECIAL PROVISIONS.

AT THE CONTRACTOR'S OPTION, AND UPON REMOVAL OF THE CAUSEWAY, THE CLASS II RIP RAP USED IN THE CAUSEWAY MAY BE PLACED AS RIP RAP SLOPE PROTECTION. SEE SPECIAL PROVISIONS FOR CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS AT STATION 10+92.00 -L-.

ASPHALT WEARING SURFACE IS INCLUDED IN ROADWAY QUANTITIES ON ROADWAY PLANS.

AT THE CONTRACTORS OPTION, PRESTRESSED CONCRETE END BENT AND BENT CAPS MAY BE SUBSTITUTED IN PLACE OF THE CAST-IN-PLACE CAPS. THE CONTRACTOR SHALL COORDINATE WITH THE RESIDENT ENGINEER TO RECEIVE REVISED PLANS AND DETAILS FROM THE STRUCTURES MANAGEMENT UNIT. THE REDESIGN AND ANY ADDITIONAL MATERIALS NEEDED WILL BE AT NO ADDITIONAL COST TO THE CONTRACTOR.



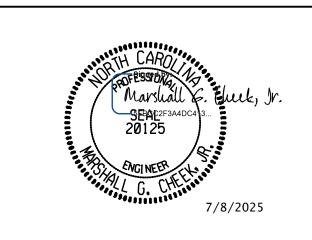
PROJECT NO. DF18311.2095167.PR

WATAUGA

10+92.00 -L-

SHEET 4 OF 5

STATION:



DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER
BEECH CREEK
ON SR 1139 BETWEEN
SR 1312 AND SR 1125

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS

201 WEST MARION ST
SUITE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C-0275

	REVI:	SIO	NS		SHEET NO.
BY:	DATE:	NO.	BY:	DATE:	S-4
		3			TOTAL SHEETS
		4			26

DRAWN BY: ZCS DATE: 5/25 CHECKED BY: MGC DATE: 6/25

	TOTAL BILL OF MATERIAL													
ITEM	CONSTRUCTION, MAINTENANCE AND REMOVAL OF TEMPORARY ACCESS	REMOVAL OF EXISTING STRUCTURE	ASBESTOS ASSESSMENT	PILE EXCAVATION IN SOIL	PILE EXCAVATION NOT IN SOIL	3'-0"Ø DRILLED PIERS IN SOIL	3'-0"Ø DRILLED PIERS NOT IN SOIL	PERMANENT STEEL CASING FOR 3'-0"Ø DRILLED PIERS	CSL TESTING	UNCLASSIFIED STRUCTURE EXCAVATION	CLASS A CONCRETE	BRIDGE APPROACH SLABS		
	LUMP SUM	LUMP SUM	LUMP SUM	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	LIN.FT.	EA.	LUMP SUM	C.Y.	LUMP SUM		
SUPERSTRUCTURE														
END BENT 1				18.20	24.80						48.5			
BENT 1						30.00	78.00	35.00	1		16.7			
END BENT 2				53.50	37.80					LUMP SUM	48.5			
TOTALS	LUMP SUM	LUMP SUM	LUMP SUM	71.70	62.60	30.00	78.00	35.00	1	LUMP SUM	113.7	LUMP SUM		

				TC	TAL BI	L OF MA	TERIAL						
ITEM	REINFORCING STEEL	SPIRAL COLUMN REINFORCING STEEL	PILE DRIVING EQUIPMENT SETUP FOR HP 12 X 53 STEEL PILES	HF S	12×53 STEEL PILES	STEEL PILE POINTS	ONE BAR METAL RAIL	1'-0" X 1-9½" CONCRETE PARAPET	RIP RAP, CLASS II	GEOTEXTILE FOR DRAINAGE	ELASTOMERIC BEARINGS	PRES CO	"× 2'-0" STRESSED NCRETE ED SLABS
	LBS.	LBS.	EA.	NO.	LIN.FT.	EA.	LIN.FT.	LIN.FT.	TONS	S.Y.	LUMP SUM	NO.	LIN.FT.
SUPERSTRUCTURE							184.13	200.29				20	1000.00
END BENT 1	5927			5	75				155	175			
BENT 1	10341	2140											
END BENT 2	5927		3	5	125	3			175	195			
TOTALS	22195	2140	3	10	200	3	184.13	200.29	330	370	LUMP SUM	20	1000.00

PROJECT NO.DF18311.2095167.PR

WATAUGA

___ COUNTY

STATION: 10+92.00 -L-

SHEET 5 OF 5



STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION
RALEIGH

GENERAL DRAWING

FOR BRIDGE OVER
BEECH CREEK
ON SR 1139 BETWEEN
SR 1312 AND SR 1125

I DOCUMENT NOT CONSIDERED FINAL	┡
UNLESS ALL SIGNATURES COMPLETED	
TGS ENGINEERS 201 W. MARION ST STE 200	ŀ
SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C-0275	Ľ

		SR	1312	2	AND	SR	112	5
			REVIS	SIO	NS			SHEET
NO.	BY:	D	ATE:	NO.	BY:	DAT	E:	S-5
~								TATAL

DRAWN BY: ZCS DATE: 5/25 CHECKED BY: MGC DATE: 6/25

		LC	DAD AND	RESIS	STANCE T	FACTOI	R RAT	ING (LRFR) Sl					SED	CON	CRETE (GIRDE T	RS	CED 44	>F 111 11	INALT CTA		<u> </u>
												ENGTH I L	_IMIT S						•	SERVIC	JE III LI	IMIT STA	(IE	4
				#					<u> </u>	10ME	NT			S	HEAF	₹				M(OMENT	<u> </u>	I	
HOAD TYPE		AEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A	1	1.54		1.75	0.26	3.57	35'	EL	13.42	0.65	1.54	35'	EL	1.71	0.80	0.26	4.53	35'	EL	16.92	
DESIC		HL-93 (OPERATING)	N/A		2.78		1.35	0.26	4.63	35'	EL	13.42	0.65	2.78	35'	EL	1.71	N/A						
LOAI)	HS-20 (INVENTORY)	36.000	2	2.10	75.60	1.75	0.26	4.53	35'	EL	13.42	0.65	2.10	35'	EL	1.71	0.80	0.26	5.94	35'	EL	13.42	
		HS-20 (OPERATING)	36.000		3.36	120.96	1.35	0.26	5.87	35'	EL	13.42	0.65	3.36	35'	EL	6.42	N/A						
		SNSH	13.500		6.44	86.94	1.4	0.26	10.08	35'	EL	13.42	0.65	6.44	35'	EL	6.42	0.80	0.26	10.35	35'	EL	16.92	
	ш	SNGARBS2	20.000		4.96	99.20	1.4	0.26	8.30	35'	EL	13.42	0.65	4.96	35'	EL	6.42	0.80	0.26	8.71	35'	EL	13.42	
	IICLE	SNAGRIS2	22.000		4.78	105.16	1.4	0.26	8.24	35'	EL	13.42	0.65	4.78	35'	EL	6.42	0.80	0.26	8.64	35'	EL	13.42	
	VE V	SNCOTTS3	27.250		3.18	88.66	1.4	0.26	5.13	35'	EL	13.42	0.65	3.18	35'	EL	6.42	0.80	0.26	5.18	35'	EL	16.92	
	S)	SNAGGRS4	34.925		2.92	101.98	1.4	0.26	4.67	35'	EL	13.42	0.65	2.92	35'	EL	6.42	0.80	0.26	4.80	35'	EL	16.92	
	SINGLE VEH (SV)	SNS5A	35.550		3.11	110.56	1.4	0.26	4.66	35'	EL	16.92	0.65	3.11	35'	EL	6.42	0.80	0.26	4.66	35'	EL	16.92	
	0)	SNS6A	39.950		2.92	116.65	1.4	0.26	4.45	35'	EL	13.42	0.65	2.92	35'	EL	6.42	0.80	0.26	4.49	35'	EL	16.92	
LEGAL		SNS7B	42.000		2.98	125.16	1.4	0.26	4.23	35'	EL	13.42	0.65	2.98	35'	EL	6.42	0.80	0.26	4.29	35'	EL	16.92	
LOAD		TNAGRIT3	33.000		3.50	115.50	1.4	0.26	5.56	35'	EL	16.92	0.65	3.50	35'	EL	6.42	0.80	0.26	5.56	35'	EL	16.92	
	~	TNT4A	33.075		3.24	107.16	1.4	0.26	5.41	35'	EL	13.42	0.65	3.24	35'	EL	6.42	0.80	0.26	5.55	35'	EL	16.92	
	CTO	TNT6A	41.600		3.12	129.79	1.4	0.26	4.79	35'	EL	13.42	0.65	3.12	35'	EL	6.42	0.80	0.26	4.86	35'	EL	16.92	
	RAC RAII ST)	TNT7A	42.000		2.97	124.74	1.4	0.26	4.78	35'	EL	13.42	0.65	2.97	35'	EL	6.42	0.80	0.26	5.02	35'	EL	13.42	
	XX 	TNT7B	42.000		2.90	121.80	1.4	0.26	4.81	35'	EL	13.42	0.65	2.90	35'	EL	6.42	0.80	0.26	4.94	35'	EL	16.92	
	TRUCK TRACTOF SEMI-TRAILER (TTST)	TNAGRIT4	43.000		2.48	106.64	1.4	0.26	4.70	35'	EL	13.42	0.65	2.48	35'	EL	1.71	0.80	0.26	4.93	35'	EL	13.42	
	-	TNAGT5A	45.000		2.85	128.25	1.4	0.26	4.56	35'	EL	13.42	0.65	2.85	35'	EL	1.71	0.80	0.26	4.62	35'	EL	16.92	
		TNAGT5B	45.000	3	2.18	98.10	1.4	0.26	4.30	35'	EL	13.42	0.65	2.18	35'	EL	1.71	0.80	0.26	4.45	35'	EL	16.92	
				T	1	1															T		T	

43.000 (4) 2.15 92.45 1.3 0.26 4.33 35' EL 13.42 0.65 2.15 35' EL 1.71 0.80 0.26 4.12 35' EL 16.92

LRFR SUMMARY FOR SPAN "A"

LOAD FACTORS:

LIMIT STATE DESIGN LOAD 1.25 1.50 STRENGTH I RATING FACTORS SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING * *

4 EMERGENCY VEHICLE LOAD RATING

* * SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

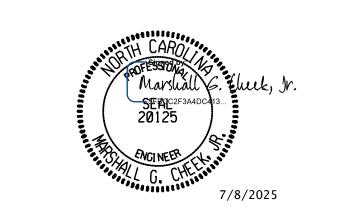
PROJECT NO. DF18311.2095167.PR

COUNTY

WATAUGA

STATION: 10+92.00 -L-

SHEET 1 OF 2



STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

LRFR SUMMARY FOR 35' CORED SLAB UNIT 60° SKEW

(NON-INTERSTATE TRAFFIC)

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS

201 WEST MARION ST
SUITE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C-0275

REVISIONS S-6 DATE: NO. BY: DATE: TOTAL SHEETS 26

05/25 05/25 MGC DATE: CHECKED BY : 06/25 DESIGN ENGINNER OF RECORD: STM DATE:

DATE:

STM

ASSEMBLED BY:

EMERGENCY

VEHICLE (EV)

EV2

EV3

STM

MGC

DESIGN ENGINNER OF RECORD: STM DATE:

ASSEMBLED BY:

CHECKED BY :

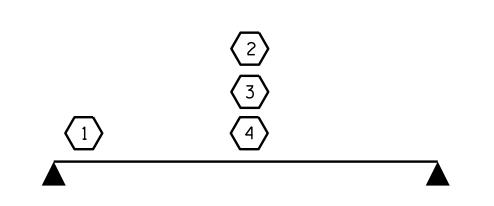
05/25 05/25

06/25

DATE:

DATE:

		L0	AD AND	RESIS	STANCE	FACT0I	R RAT	ING (LRFR) Sl	JMMAF	RY FOR	PRES	TRES	SED	CONG	CRETE (GIRDE	RS					
											STR	ENGTH I L	_IMIT ST	TATE						SERVIO	CE III LI	MIT STA	ГЕ	
				#					ľ	10МЕ	NT			S	HEAR					М	OMENT	-		
I OAD TYPE		VEHICLE	WEIGHT (W) (TONS)	CONTROLLING LOAD RATING	MINIMUM RATING FACTORS (RF)	TONS = W × RF	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	LIVE-LOAD FACTORS (DISTRIBUTION FACTORS (DF)	RATING FACTOR	SPAN	GIRDER LOCATION	DISTANCE FROM LEFT END OF SPAN (ft)	COMMENT NUMBER
		HL-93 (INVENTORY)	N/A		1.06		1.75	0.26	1.21	65'	EL	31.92	0.66	1.06	65'	EL	1.62	0.80	0.26	1.32	65'	EL	31.92	
DESIG		HL-93 (OPERATING)	N/A		1.57		1.35	0.26	1.57	65'	EL	31.92		1.67		EL	5.92	N/A						
LOAI	ט	HS-20 (INVENTORY)	36.000	2	1.56	56.16	1.75	0.26	1.56	65'	EL	31.92	0.66	1.57	65'	EL	5.92	0.80	0.26	1.69	65'	EL	31.92	
		HS-20 (OPERATING)	36.000		2.02	72.72	1.35	0.26	2.02	65'	EL	31.92	0.66	2.08	65'	EL	5.92	N/A						
		SNSH	13.500		3.71	50.09	1.4	0.26	4.27	65'	EL	31.92	0.66	4.88	65'	EL	5.92	0.80	0.26	3.71	65'	EL	31.92	
	Щ	SNGARBS2	20.000		2.81	56.20	1.4	0.26	3.23	65'	EL	31.92	0.66	3.45	65'	EL	5.92	0.80	0.26	2.81	65'	EL	31.92	
		SNAGRIS2	22.000		2.68	58.96	1.4	0.26	3.08	65'	EL	31.92	0.66	3.20	65'	EL	5.92	0.80	0.26	2.68	65'	EL	31.92	
	E VEI (SV)	SNCOTTS3	27.250		1.85	50.41	1.4	0.26	2.13	65'	EL	31.92	0.66	2.36	65'	EL	5.92	0.80	0.26	1.85	65'	EL	31.92	
		SNAGGRS4	34.925		1.56	54.48	1.4	0.26	1.80	65'	EL	31.92	0.66	1.96	65'	EL	5.92	0.80	0.26	1.56	65'	EL	31.92	
	SING	SNS5A	35.550		1.53	54.39	1.4	0.26	1.76	65'	EL	31.92	0.66	1.99	65'	EL	5.92	0.80	0.26	1.53	65'	EL	31.92	
	01	SNS6A	39.950		1.41	56.33	1.4	0.26	1.62	65'	EL	31.92	0.66	1.81	65'	EL	5.92	0.80	0.26	1.41	65'	EL	31.92	
LEGAL		SNS7B	42.000		1.34	56.28	1.4	0.26	1.54	65'	EL	31.92	0.66	1.79	65'	EL	5.92	0.80	0.26	1.34	65'	EL	31.92	
LOAD		TNAGRIT3	33.000		1.72	56.76	1.4	0.26	1.98	65'	EL	31.92	0.66	2.18	65'	EL	5.92	0.80	0.26	1.72	65'	EL	31.92	
	~	TNT4A	33.075		1.73	57.22	1.4	0.26	1.99	65'	EL	31.92	0.66	2.11	65'	EL	5.92	0.80	0.26	1.73	65'	EL	31.92	
	CTO	TNT6A	41.600		1.42	59.07	1.4	0.26	1.63	65'	EL	31.92	0.66	1.94	65'	EL	5.92	0.80	0.26	1.42	65'	EL	31.92	
	RA(ST)	TNT7A	42.000		1.43	60.06	1.4	0.26	1.65	65'	EL	31.92	0.66	1.86	65'	EL	5.92	0.80	0.26	1.43	65'	EL	31.92	
	JCK TRACTOR EMI-TRAILER (TTST)	TNT7B	42.000		1.49	62.58	1.4	0.26	1.71	65'	EL	31.92	0.66	1.74	65'	EL	5.92	0.80	0.26	1.49	65'	EL	31.92	
	SEN	TNAGRIT4	43.000		1.41	60.63	1.4	0.26	1.62	65'	EL	31.92	0.66	1.67	65'	EL	5.92	0.80	0.26	1.41	65'	EL	31.92	
		TNAGT5A	45.000		1.33	59.85	1.4	0.26	1.53	65'	EL	31.92	0.66	1.68	65'	EL	5.92	0.80	0.26	1.33	65'	EL	31.92	
		TNAGT5B	45.000	(3)	1.31	58.95	1.4	0.26	1.50	65'	EL	31.92	0.66	1.58	65'	EL	5.92	0.80	0.26	1.31	65'	EL	31.92	
EMERG	ENCY	EV2	28.750		1.99	57.21	1.3	0.26	2.47	65'	EL	31.92	0.66	2.57	65'	EL	5.92	0.80	0.26	1.99	65'	EL	31.92	
VEHICL		EV3	43.000	4	1.30	55.90	1.3	0.26	1.61	65'	EL	31.92	0.66	1.69	65'	EL	5.92	0.80	0.26	1.30	65'	EL	31.92	



LRFR SUMMARY FOR SPAN "B"

LOAD FACTORS:

LIMIT STATE DESIGN LOAD 1.25 1.50 STRENGTH I RATING FACTORS SERVICE III 1.00 1.00

NOTES:

MINIMUM RATING FACTORS ARE BASED ON THE STRENGTH I AND SERVICE III LIMIT STATES.

ALLOWABLE STRESSES FOR SERVICE III LIMIT STATE ARE AS REQUIRED FOR DESIGN.

COMMENTS:

CONTROLLING LOAD RATING

1 DESIGN LOAD RATING (HL-93)

2 DESIGN LOAD RATING (HS-20)

(3) LEGAL LOAD RATING * *

4 EMERGENCY VEHICLE LOAD RATING

* * SEE CHART FOR VEHICLE TYPE

GIRDER LOCATION

I - INTERIOR GIRDER

EL - EXTERIOR LEFT GIRDER

ER - EXTERIOR RIGHT GIRDER

PROJECT NO. DF18311.2095167.PR

WATAUGA

COUNTY

STATION: 10+92.00 -L-

SHEET 2 OF 2

7/8/2025

STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

LRFR SUMMARY FOR 65' CORED SLAB UNIT 60° SKEW

(NON-INTERSTATE TRAFFIC)

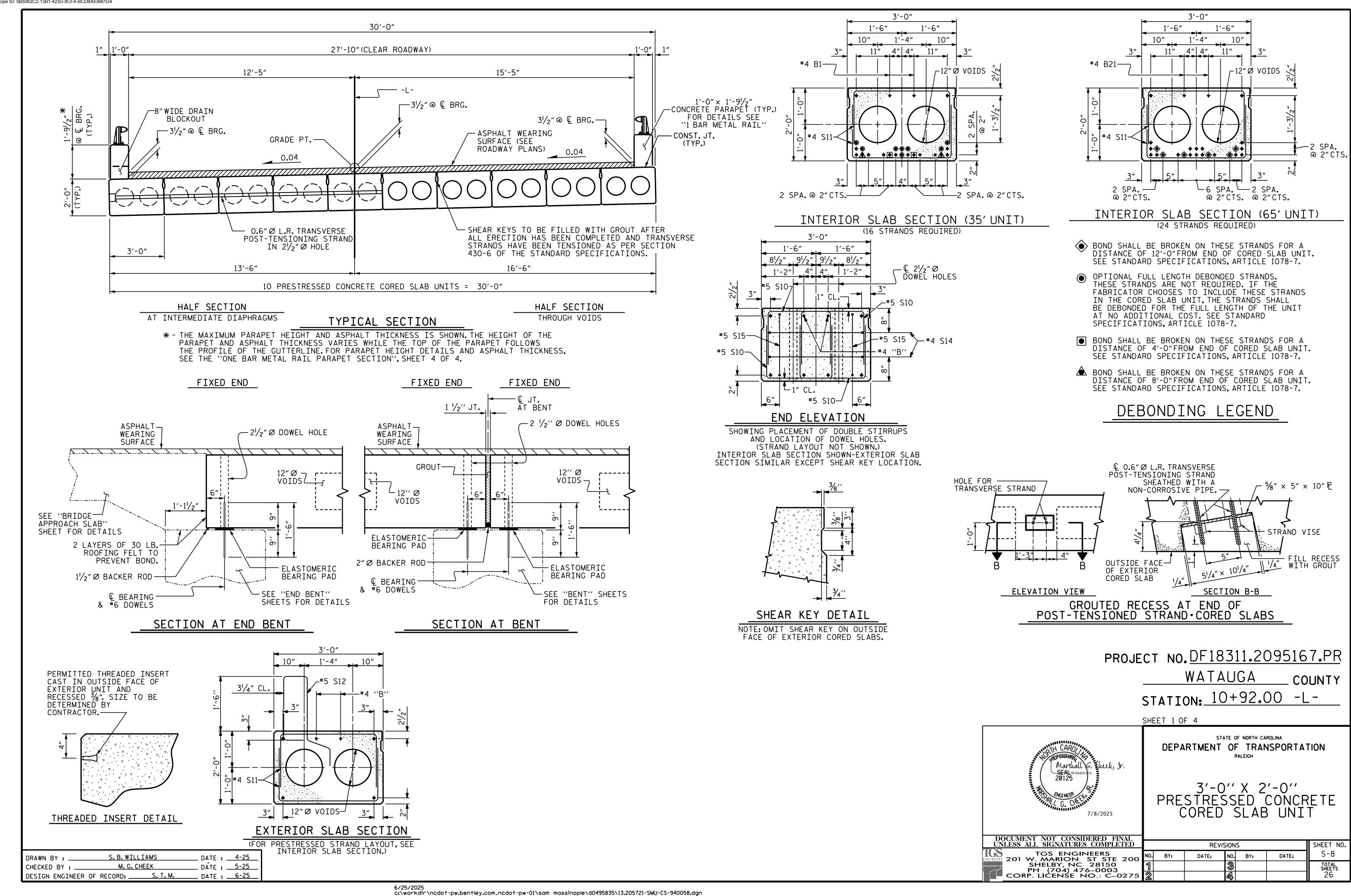
DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

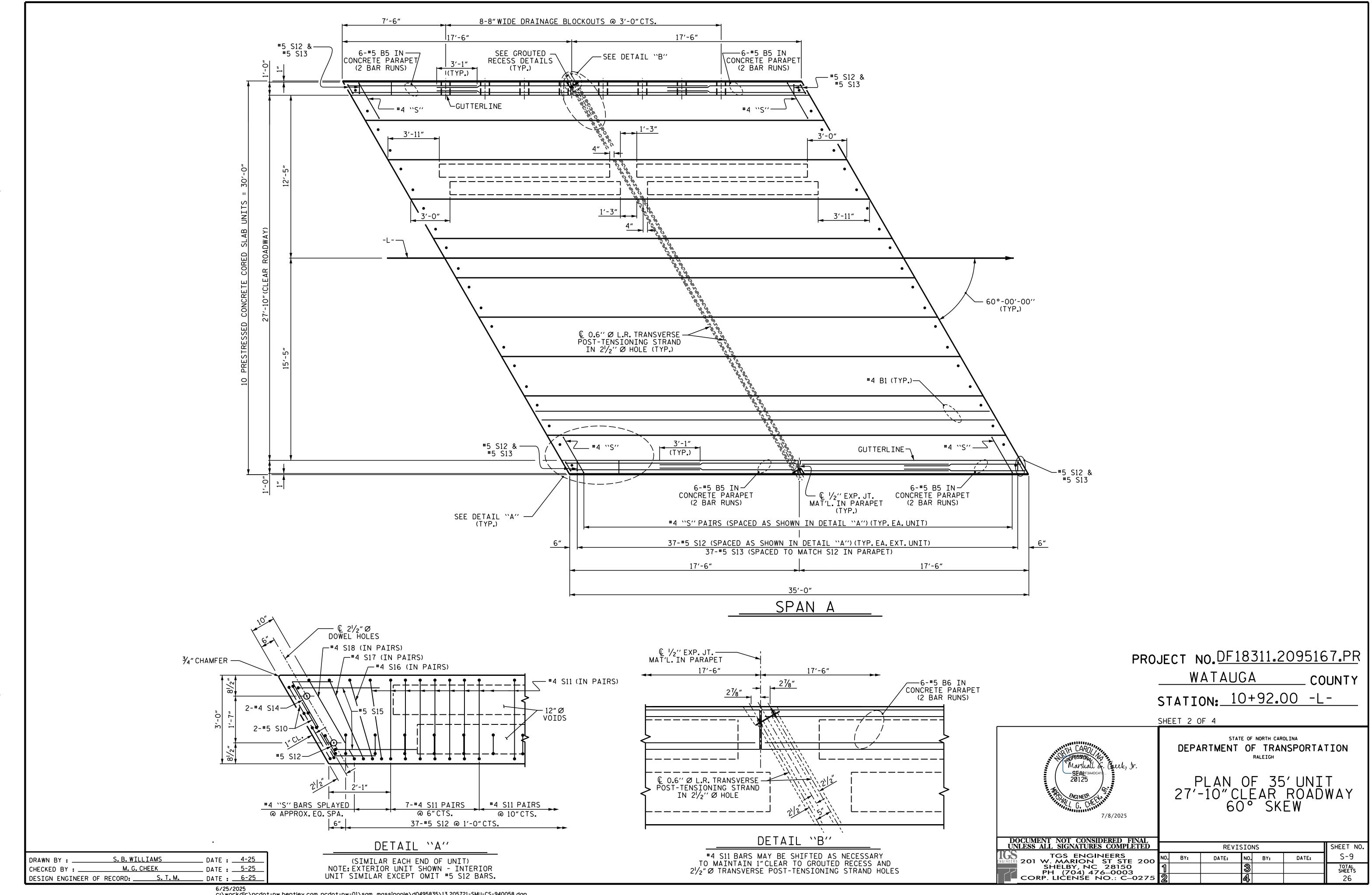
TGS ENGINEERS

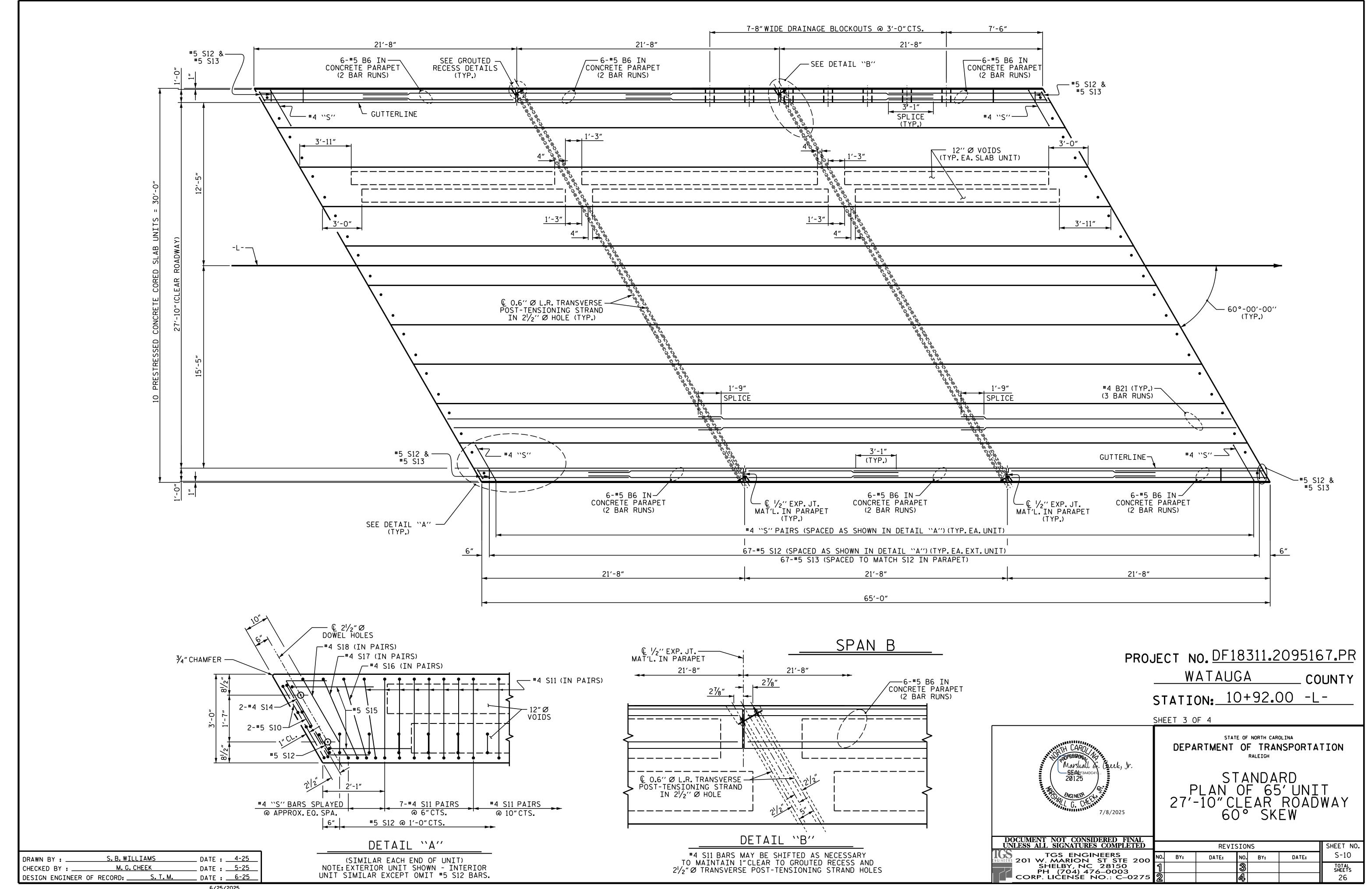
201 WEST MARION ST
SUITE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C-0275

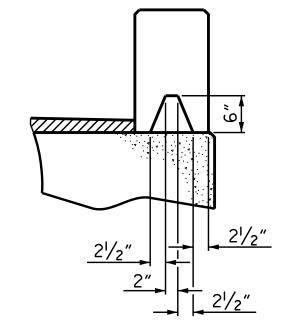
REVISIONS S-7 DATE: NO. BY: DATE: BY: TOTAL SHEETS 26

6/25/2025
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SECTION S-S AT DAM IN OPEN JOINT (THIS IS TO BE USED ONLY WHEN SLIP FORM IS USED)

CHAMFER

CHAMFER

-4" OPENING

€ 1/2"EXP.JT.MAT'L HELD IN

PLACE WITH GALVANIZED NAILS.

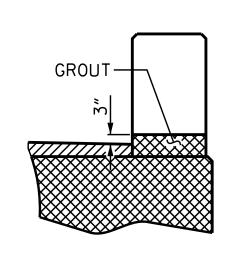
(NOTE: OMIT EXP. JT. MAT'L.

¾″ **I**CHAMFER

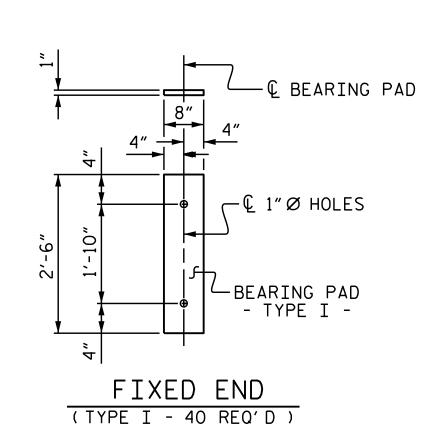
© OPEN JT. IN PARAPET @ BENT →

HAMFE

WHEN SLIP FORM IS USED) -

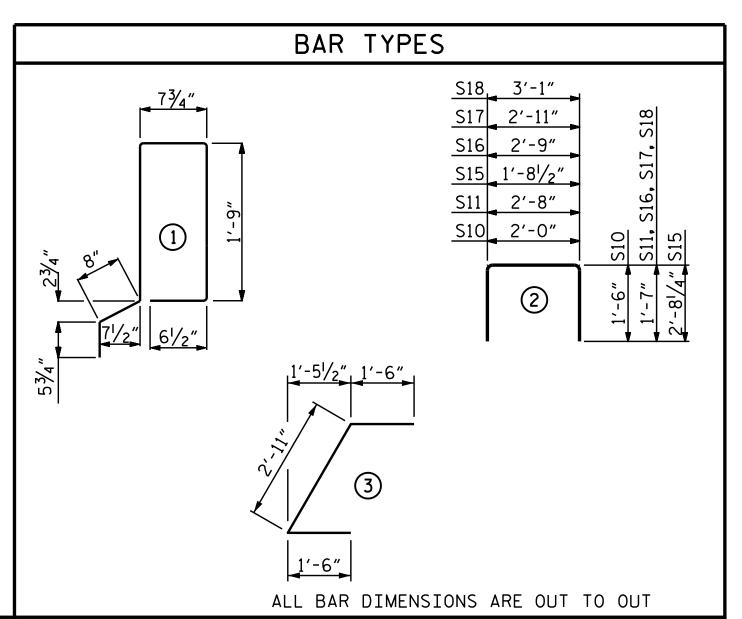


SECTION T-T AT OPEN JOINT AT BENT (THIS IS TO BE USED WHERE FOAM JOINT IS NOT USED)



ELASTOMERIC BEARING DETAILS

ELASTOMER IN ALL BEARINGS SHALL BE 60 DUROMETER HARDNESS.



BILL OF MATERIAL FOR ONE 35' CORED SLAB UNIT								BILL OF MATERIAL FOR ONE 65' CORED SLAB UNIT							
				EXTERI	OR UNIT	INTERI(OR UNIT					EXTERI(OR UNIT	INTERI	OR UNIT
BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT	BAR	NUMBER	SIZE	TYPE	LENGTH	WEIGHT	LENGTH	WEIGHT
B1	2	#4	STR	34'- 8"	46	34'- 8"	46	B21	6	#4	STR	22'-10"	92	22'-10"	92
S10	8	#5	2	5′-0″	42	5′-0″	42	S10	8	#5	2	5′-0″	42	5′-0″	42
S11	86	#4	2	5′-10″	335	5′-10″	335	S11	158	#4	2	5′-10″	616	5′-10″	616
* S12	39	#5	1	5′-10″	237			* S12	69	#5	1	5′-10″	420		
S14	4	#4	3	5′-11″	16	5′-11″	16	S14	4	#4	3	5′-11″	16	5′-11″	16
S15	4	# 5	2	7'-1"	30	7'-1"	30	S15	4	#5	2	7'-1"	30	7′-1″	30
S16	4	#4	2	5′-11″	16	5′-11″	16	S16	4	#4	2	5′-11″	16	5′-11″	16
S17	4	#4	2	6′-1″	16	6′-1″	16	S17	4	#4	2	6′-1"	16	6'-1"	16
S18	4	#4	2	6′-3"	17	6′-3″	17	S18	4	#4	2	6′-3″	17	6′-3″	17
REINFO	ORCING S	STEEL	LBS	5.	518		518	REINF	ORCING S	STEEL	LBS) .	845		845
	(Y COATE NFORCINO		LB:	5.	237				Y COATE IFORCING		LB:	5.	420		
	P.S.I.CO			6.2		6.	2		P.S.I. CO		CU. YDS		11.2		11.2
0.6" Ø	L.R. STR	ANDS	No.	16		16	ŝ	0.6"Ø	L.R. STR	ANDS	No).	24		24

ELEVATION AT EXPANSION JOINTS 2¹/₄" CL.

ONE DAD METAL	
ONE BAR METAL	
RAIL PARAPET SECTION	

8" WIDE-

DRAIN BLOCKOUT

(HEIGHT VARIES)

THE DRAIN OPENING AT THE GUTTERLINE SHALL BE 4" X 8". THE HEIGHT OF THE BLOCKOUT IN THE PARAPET SHALL EXTEND FROM THE TOP OF THE CORED SLAB UNIT TO THE TOP OF THE DRAIN OPENING.

CONCRETE	RELEA	ASE	STRENGTH
UNIT			PSI
35' UNITS			4000
65' UNITS			4800

DRAWN BY :	S.B.WIL	LIAMS	DATE : _	4-25
CHECKED BY :	M. G. (CHEEK	DATE : _	5-25
DESIGN ENGINEER		S.T.M.	DATE : _	6-25

CORED	SLABS	S REQ	UIRED
35' UNIT	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR C.S.	2	35′-0″	70′-0″
INTERIOR C.S.	8	35′-0″	280'-0"
TOTAL	10		350'-0"
CORED	SLABS	S REQ	UIRED
65' UNIT	NUMBER	LENGTH	TOTAL LENGTH
EXTERIOR C.S.	2	65′-0″	130'-0"
INTERIOR C.S.	8	65′-0″	520′-0″
TOTAL	10		650'-0"

1		
	ASPHALT OVERLAY THICKNESS @ MID-SPAN	PARAPET HEIGHT @ MID-SPAN
35' UNITS	27/8"	1′-87/8″
65' UNITS	2¾6″	1′-8 <mark>¾</mark> 6″

43,950

GRADE 270 S	TRANDS
	0.6"Ø L.R.
AREA (SQUARE INCHES)	0.217
ULTIMATE STRENGTH	58,600

(LBS. PER STRAND

APPLIED PRESTRESS

(LBS.PER STRAND

DEAD LOAD DEFLECTION AND CAMBER		
	35' UNIT	65' UNIT
	3'-0" × 2'-0"	3'-0" × 2'-0"
	0.6″Ø L.R. STRAND	0.6″Ø L.R. STRAND
CAMBER (SLAB ALONE IN PLACE)	¹ / ₁₆ " ∤	1¾″ ੈ
DEFLECTION DUE TO SUPERIMPOSED DEAD LOAD***	¹ ⁄16″ ∤	7⁄16″ ∤
FINAL CAMBER	5⁄8″ ∤	1 ⁵ ⁄ ₁₆ " ∤

** INCLUDES FUTURE WEARING SURFACE

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ALL REINFORCING STEEL CAST WITH THE CORED SLAB SECTIONS SHALL BE GRADE 60 AND SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE CORED SLABS.

RECESSES FOR TRANSVERSE STRANDS SHALL BE GROUTED AFTER THE TENSIONING OF THE STRANDS.

THE $2^{1}\!/_{2}$ " Ø DOWEL HOLES AT FIXED ENDS OF SLAB SECTIONS SHALL BE FILLED WITH GROUT. THE $2^{1}\!/_{2}$ " Ø DOWEL HOLES AT EXPANSION ENDS OF SLAB SECTIONS SHALL BE FILLED WITH JOINT SEALER MATERIAL TO $1\frac{1}{2}$ ABOVE THE TOP OF DOWELS AND THEN FILLED WITH GROUT.

THE JOINT SEALER MATERIAL SHALL CONFORM TO THE REQUIREMENTS OF TYPE SL LOW MODULUS SILICONE SEALANT. THE 2" Ø BACKER ROD SHALL CONFORM TO THE REQUIREMENTS OF TYPE M BOND BREAKER. SEE SECTION 1028 OF THE STANDARD SPECIFICATIONS.

WHEN CORED SLABS ARE CAST, AN INTERNAL HOLD-DOWN SYSTEM SHALL BE EMPLOYED TO PREVENT VOIDS FROM RISING OR MOVING SIDEWAYS. AT LEAST SIX WEEKS PRIOR TO CASTING CORED SLABS, THE CONTRACTOR SHALL SUBMI TO THE ENGINEER FOR REVIEW AND COMMENT, DETAILED DRAWINGS OF THE PROPOSED HOLD-DOWN SYSTEM. IN ADDITION TO STRUCTURAL DETAILS, LOCATION AND SPACING OF THE HOLD-DOWNS SHALL BE INDICATED.

ALL REINFORCING STEEL IN PARAPET SHALL BE EPOXY COATED.

PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE CORED SLAB UNIT ENDS.

APPLY EPOXY PROTECTIVE COATING TO CORED SLAB UNIT ENDS.

GROOVED CONTRACTION JOINTS, $\frac{1}{2}$ " IN DEPTH, SHALL BE TOOLED IN ALL EXPOSED FACES OF THE PARAPET AND IN ACCORDANCE WITH ARTICLE 825-10(B) OF THE STANDARD SPECIFICATIONS. A CONTRACTION JOINT SHALL BE LOCATED AT EACH THIRD POINT BETWEEN PARAPET EXPANSION JOINTS. ONLY ONE CONTRACTION JOINT IS REQUIRED AT MIDPOINT OF PARAPET SEGMENTS LESS THAN 20 FEET IN LENGTH AND NO CONTRACTION JOINTS ARE REQUIRED FOR THOSE SEGMENTS LESS THAN 10 FEET IN LENGTH.

FLAME CUTTING OF THE TRANSVERSE POST-TENSIONING STRAND IS NOT ALLOWED.

MAINTAIN A SYMMETRIC TENSION FORCE BETWEEN EACH PAIR OF TRANSVERSE POST TENSIONING STRANDS IN THE DIAPHRAGM.

THE #4 S11 STIRRUPS MAY BE SHIFTED AS NECESSARY TO MAINTAIN 1" CLEAR TO THE GROUTED RECESS.

THE PERMITTED THREADED INSERTS ARE DETAILED AS AN OPTION FOR THE CONTRACTOR TO ATTACH FALSEWORK AND FORMWORK DURING CONSTRUCTION.

THE PERMITTED THREADED INSERTS IN THE EXTERIOR UNITS SHALL BE SIZED BY THE CONTRACTOR, SPACED AT 4'-O"CENTERS AND GALVANIZED IN ACCORDANCE WITH SECTION 1076 OF THE STANDARD SPECIFICATIONS. STAINLESS STEEL THREADED INSERTS MAY BE USED AS AN ALTERNATE.

THE PERMITTED THREADED INSERTS SHALL BE GROUTED BY THE CONTRACTOR IMMEDIATELY FOLLOWING REMOVAL OF THE FALSEWORK.

THE COST OF THE PERMITTED THREADED INSERTS SHALL BE INCLUDED IN THE PRICE BID FOR THE PRECAST UNITS.

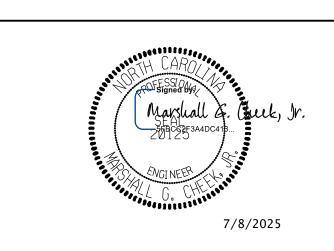
APPLY EPOXY PROTECTIVE COATING TO EXTERIOR FACE OF THE EXTERIOR CORED SLAB UNITS THAT REQUIRE DRAINS IN THE PARAPET.

THE BOTTOM TWO #5 "B" BARS IN THE PARAPET MAY BE FIELD CUT TO AVOID DRAINS.

> PROJECT NO. <u>DF18311.2095167.PR</u> WATAUGA

__ COUNTY STATION: 10+92.00 -L-

SHEET 4 OF 4



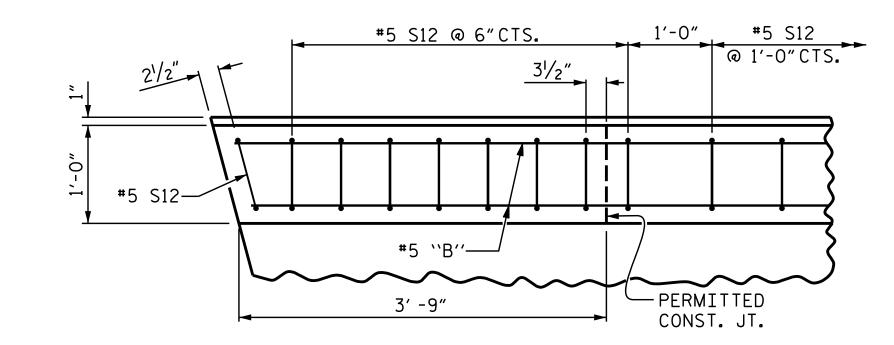
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION

3'-0" X 2'-0"
PRESTRESSED CONCRETE
CORED SLAB UNIT

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 201 W. MARION ST STE 200 SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275 **REVISIONS**

SHEET NO S-11 DATE: DATE: BY: BY: TOTAL SHEETS





1'-0"

#6 F2-J

CONST.JT.__

#5 S12—

#7 E1 BARS ---

2¹/₄'' CL. **| 1** = =

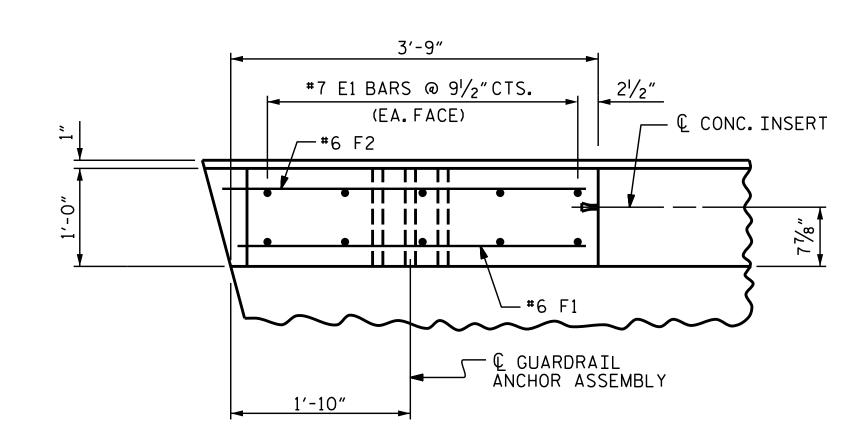
PLAN OF PARAPET

-PERMITTED

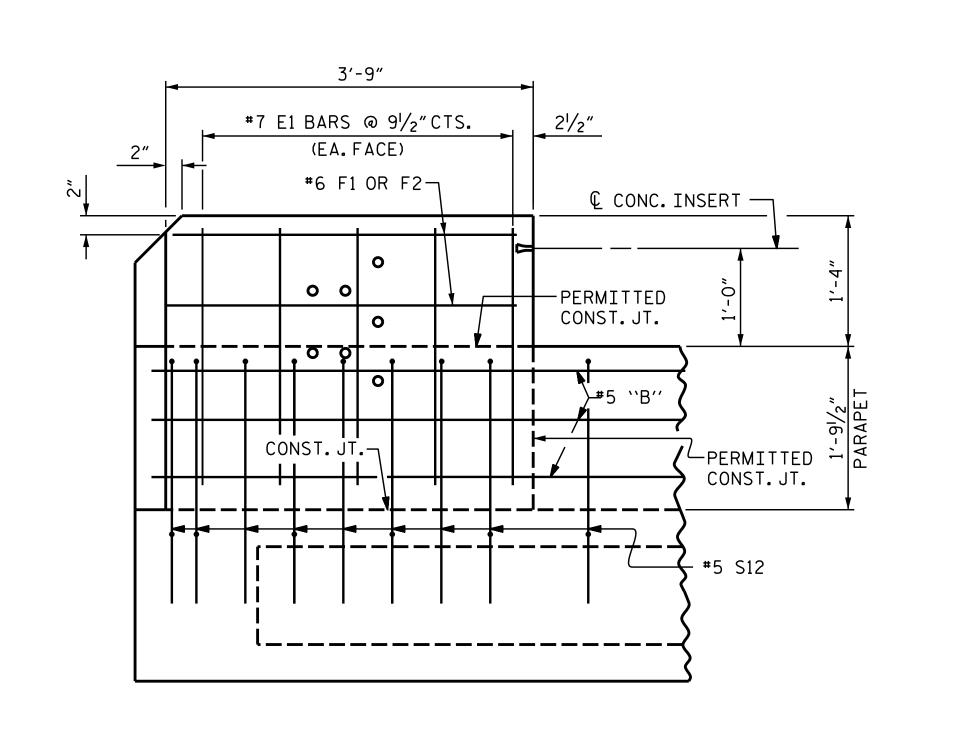
CONST.JT.

_ C GUARDRAIL

ANCHOR ASSEMBLY



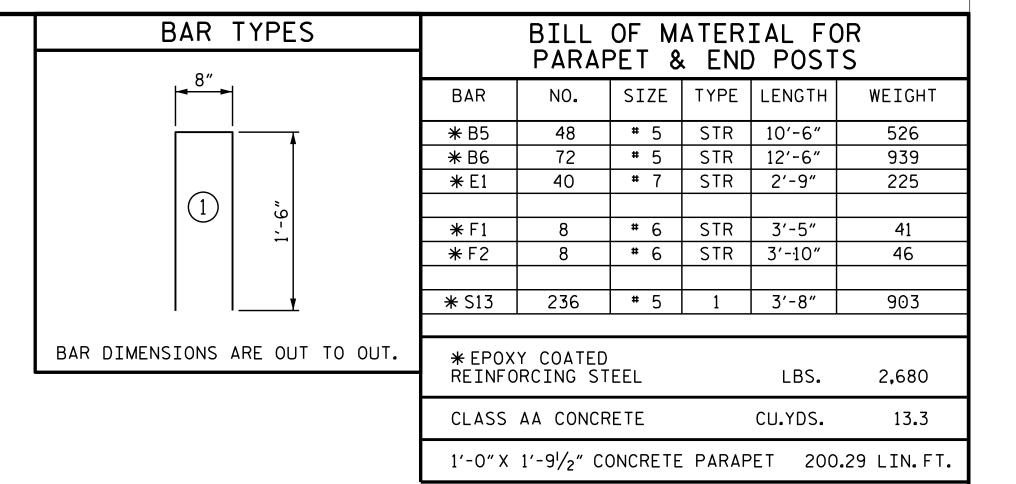
PLAN OF END POST



END VIEW

ELEVATION

PARAPET AND END POST FOR ONE BAR RAIL



NOTES:

QUANTITIES FOR THE #5 S12 ARE SHOWN IN THE CORED SLAB BILL OF MATERIAL.

PROJECT NO. <u>DF18311.2095167.PR</u>

WATAUGA COUNTY

STATION: 10+92.00 -L-

SHEET 1 OF 3

Marshall & Cleuk, Jr.

SERECTE 3A4DC413
20125

NGINEER

7/8/2025

STATE OF NORTH CAROLINA

DEPARTMENT OF TRANSPORTATION

RALEIGH

END POST DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

TGS ENGINEERS
201 W. MARION ST STE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C-0275

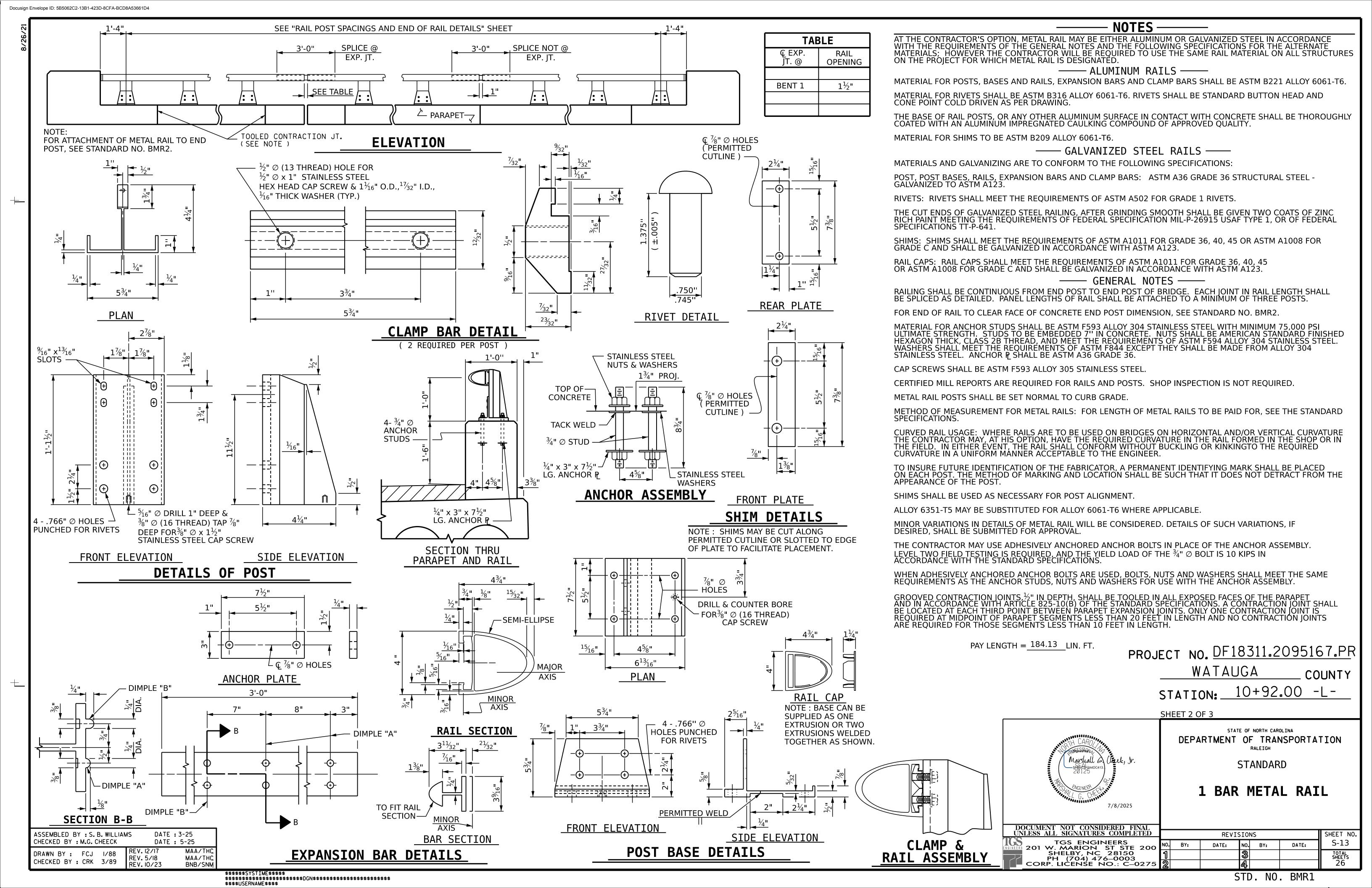
REVISIONS

10. BY: DATE: NO. BY: DATE: S-12

1 3 TOTAL SHEETS
2 4 2 26

DRAWN BY : S. B. WILLIAMS
CHECKED BY : M.G. CHEEK

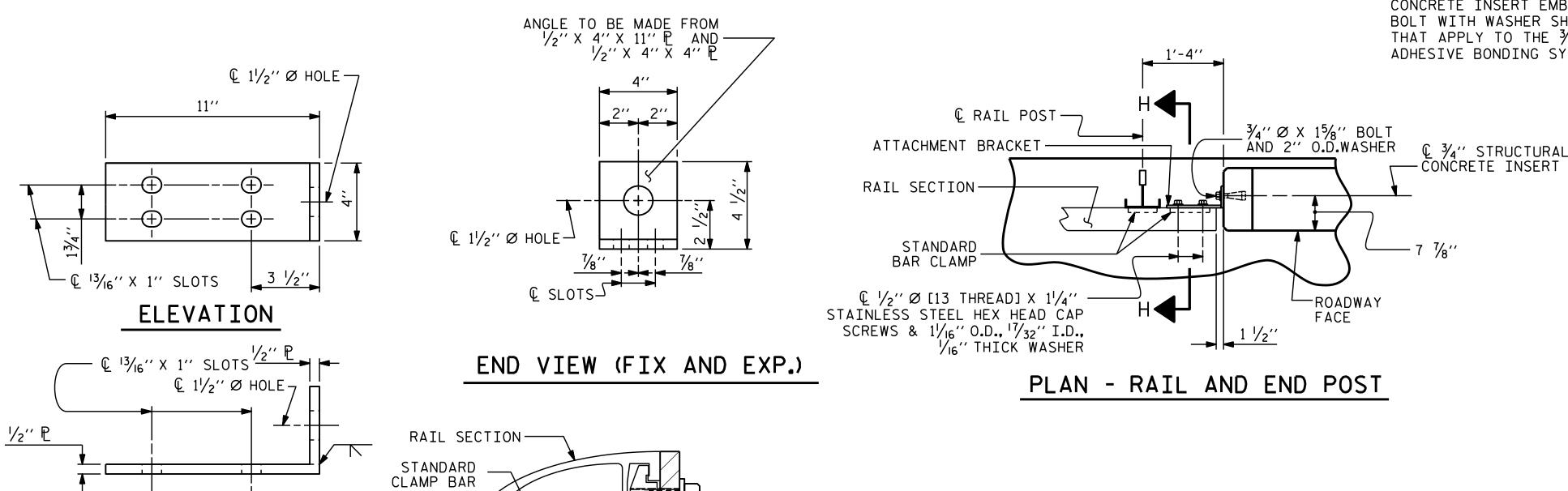
DATE : 4-25
DATE : 5-25



92'-01/8" 19 RAIL POSTS (SPACED AS SHOWN) 2 SPA. @ 4 SPA. @ 6'-6" CTS. 3′-6¾" 5'-9" CTS. 5 SPA. @ 6'-6"CTS. 3'-3" 3'-3" 3'-9" .'-4" 2'-2"CTS. 3′-9" END POST END POST

PLAN OF RAIL POST SPACINGS

(LEFT SIDE SHOWN, RIGHT SIDE SIMILAR)



 $\mathbb{Q} \frac{1}{2}$ " Ø [13 THREAD] X $\frac{1}{4}$ "

STAINLESS STEEL HEX

HEAD CAP SCREWS & 11/16" O.D., 17/32" I.D., 1/16" THICK WASHER

SECTION H-H (FIX)

FIXED

ASSEMBLED BY : S. B. WILLIAM CHECKED BY : M. G. CHEECK	MS DATE: 3-25 DATE: 5-25
IURAWN DI : FLJ 1700	REV. 5/1/06 TLA/GM REV. 10/1/11 MAA/GM REV. 12/17 MAA/THC

3 3/4′′

TOP VIEW

DETAILS FOR ATTACHING METAL RAIL TO END POST

NOTES

STRUCTURAL CONCRETE INSERT

- THE STRUCTURAL CONCRETE INSERT ASSEMBLY SHALL CONSIST OF THE FOLLOWING COMPONENTS:
- A. FERRULES SHALL BE MADE FROM STEEL MEETING THE REQUIREMENTS OF AASHTO M169, GRADE 12L14 AND SHALL HAVE A MINIMUM LENGTH OF THREADS OF $1\frac{1}{2}$ ".
- B. 1 $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH WASHER.BOLT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307.BOLT AND WASHER SHALL BE GALVANIZED. (AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLT AND WASHER MAY BE USED AS AN ALTERNATE FOR THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " GALVANIZED BOLT AND WASHER.THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.)
- C. WIRE STRUT SHOWN IN THE CONCRETE INSERT ASSEMBLY DETAIL IS THE MINIMUM ALLOWABLE SIZE AND SHALL HAVE A MINIMUM TENSILE STRENGTH OF 100,000 PSI. AS AN OPTION, A 7_{16} " Ø WIRE STRUT WITH A MINIMUM TENSILE STRENGTH OF 90,000 PSI IS ACCEPTABLE.

NOTES

METAL RAIL TO END POST CONNECTION

THE METAL RAIL TO END POST CONNECTION SHALL CONSIST OF THE FOLLOWING COMPONENTS:

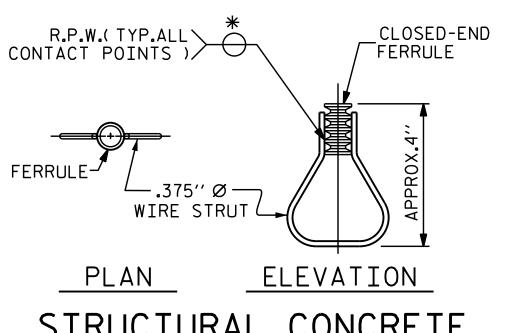
- A. 1/2" PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND SHALL BE GALVANIZED AFTER FABRICATION.
- B. $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT SHALL HAVE A WORKING LOAD SHEAR CAPACITY OF 4800 LBS. THE FERRULES SHALL ENGAGE A $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT WITH 2" O.D. WASHER IN PLACE. THE $\frac{3}{4}$ " Ø X $1\frac{5}{8}$ " BOLT SHALL HAVE N. C. THREADS.
- C. CAP SCREWS FOR RAIL ATTACHMENT TO ANGLE SHALL CONFORM TO THE REQUIREMENTS OF ASTM F593 ALLOY 305 STAINLESS STEEL. CAP SCREWS TO BE CENTERED IN SLOTS AT 60°F.
- D. STANDARD CLAMP BARS (SEE METAL RAIL SHEET).
- E. 1/2" Ø PIPE SLEEVES (IF REQUIRED) TO BE GALVANIZED.

THE COST OF THE STANDARD CLAMP BARS AND CAP SCREWS USED IN THE METAL RAIL TO END POST CONNECTION SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR LINEAR FEET OF 1 OR 2 BAR METAL RAILS.

THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT WITH BOLT SHALL BE ASSEMBLED IN THE SHOP.

THE COST OF THE $\frac{3}{4}$ " STRUCTURAL CONCRETE INSERT ASSEMBLY, AND THE $\frac{1}{2}$ " PLATES COMPLETE IN PLACE SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE CONTRACTOR, AT HIS OPTION, MAY USE AN ADHESIVE BONDING SYSTEM IN LIEU OF THE STRUCTURAL CONCRETE INSERT EMBEDDED IN THE END POST. IF THE ADHESIVE BONDING SYSTEM IS USED, THE $\frac{3}{4}$ " $\frac{3}{4}$ " $\frac{3}{4}$ " BOLT WITH WASHER SHALL BE REPLACED WITH A $\frac{3}{4}$ " $\frac{3}{4}$ " BOLT AND 2" O.D. WASHER. ALL SPECIFICATIONS THAT APPLY TO THE $\frac{3}{4}$ " $\frac{3}{4}$ " $\frac{3}{4}$ " BOLT SHALL APPLY TO THE $\frac{3}{4}$ " BOLT. FIELD TESTING OF THE ADHESIVE BONDING SYSTEM IS NOT REQUIRED.



* EACH WELDED ATTACHMENT OF WIRE TO FERRULE SHALL DEVELOP THE TENSILE STRENGTH OF THE WIRE.

PROJECT NO. DF18311.2095167.PR

WATAGUA COUNTY

STATION: 10+92.00 -L-

SHEET 3 OF 3



DEPARTMENT OF TRANSPORTATION

RALEIGH

STANDARD

END OF RAIL DETAILS

FOR ONE BAR METAL RAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

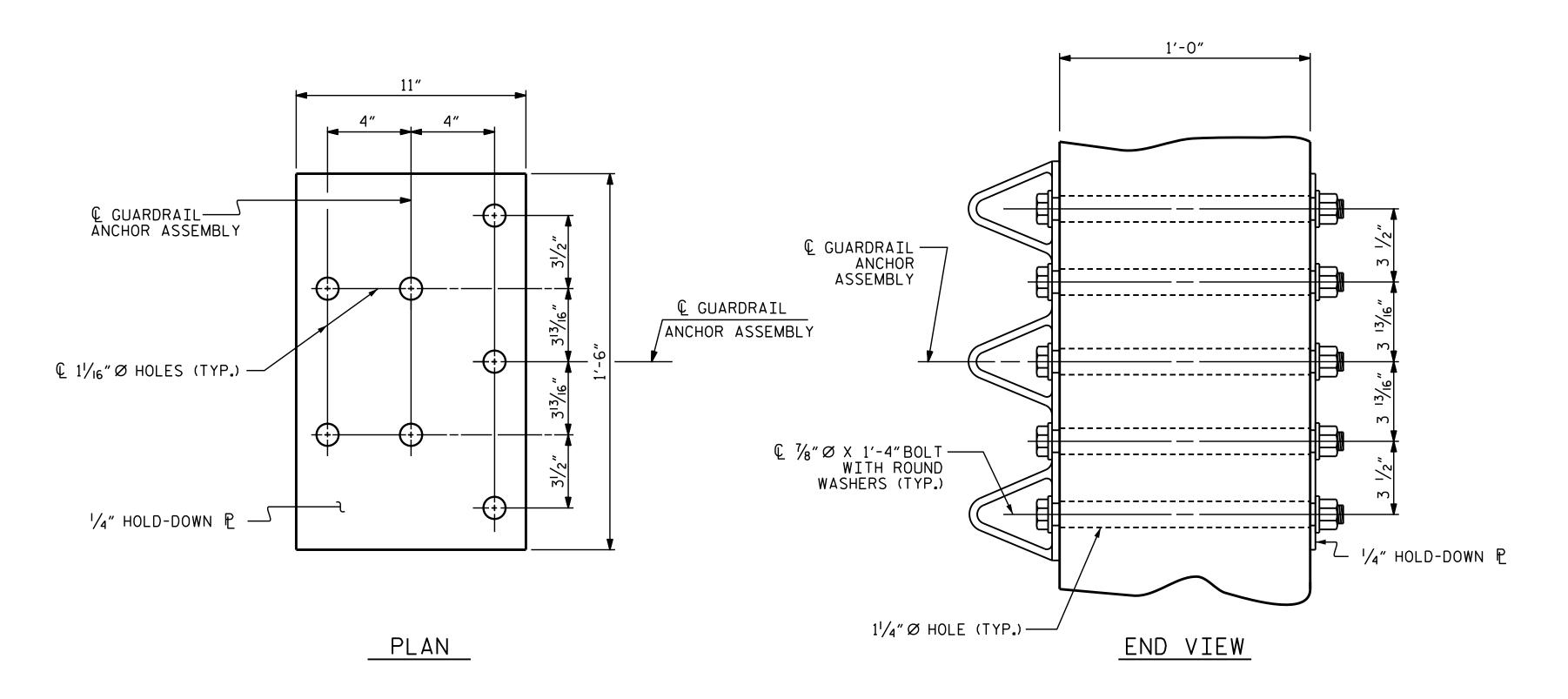
REVISIONS

SHEET NO

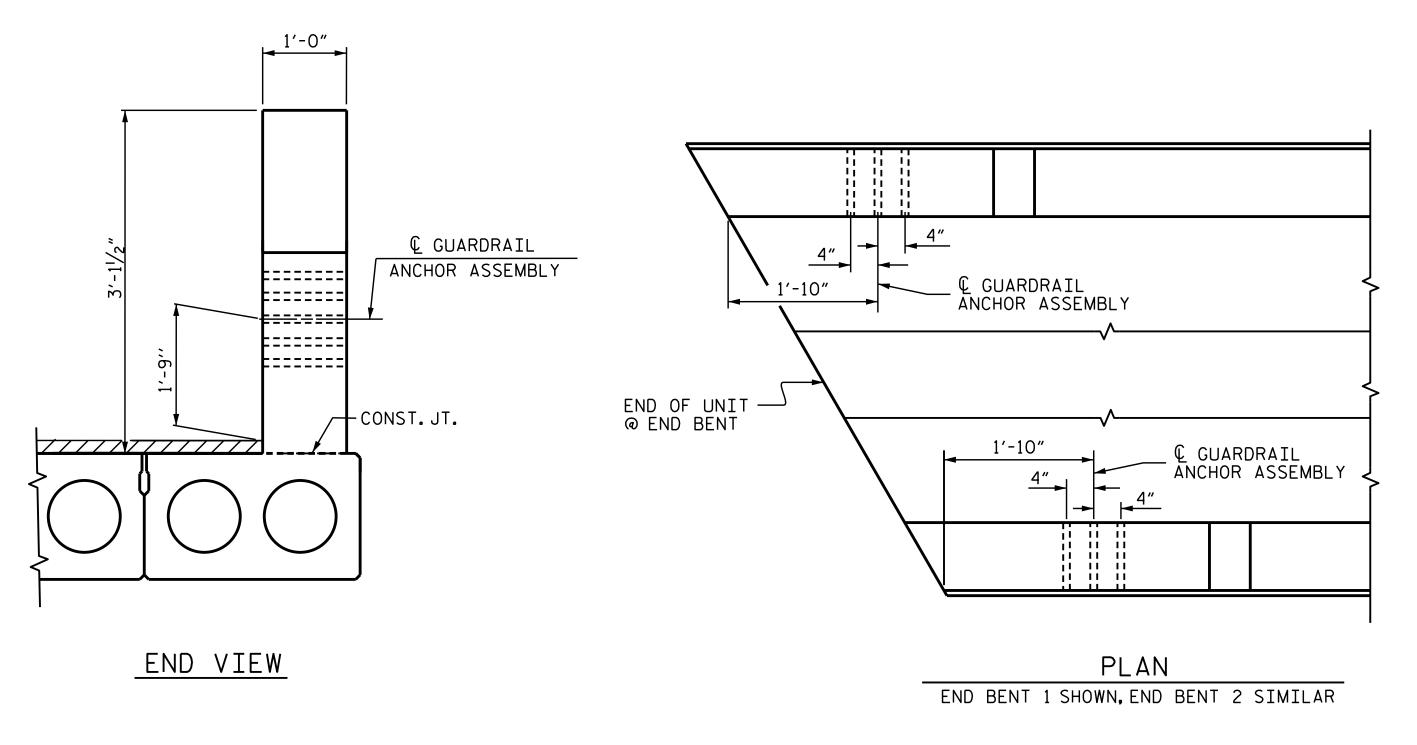
TGS ENGINEERS
201 W. MARION ST STE 200
SHELBY, NC 28150
PH (704) 476-0003
CORP. LICENSE NO.: C-0275

TOTAL
SHEETS
26

STD. NO. BMR2



GUARDRAIL ANCHOR ASSEMBLY DETAILS



LOCATION OF GUARDRAIL ANCHOR AT END POST

ASSEMBLED BY: S.B. WILLIAMS DATE: 5-25 CHECKED BY: M.G. CHEEK DATE: 5-25 REV. 1/15 REV. 12/17 REV. 5/18 DRAWN BY: MAA 5/10 MAA/THC CHECKED BY : GM 5/10 MAA/THC

NOTES

THE GUARDRAIL ANCHOR ASSEMBLY SHALL CONSIST OF A $\frac{1}{4}$ " HOLD DOWN PLATE AND 7 - $\frac{7}{8}$ " Ø BOLTS WITH NUTS AND WASHERS.

THE HOLD-DOWN PLATE SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER FABRICATION, THE HOLD-DOWN PLATE SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH AASHTO M111.

BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM A307 AND NUTS SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M291. BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED. AT THE CONTRACTOR'S OPTION, STAINLESS STEEL BOLTS, NUTS AND WASHERS MAY BE USED AS AN ALTERNATE FOR THE 1/8" Ø GALVANIZED BOLTS, NUTS AND WASHERS. THEY SHALL CONFORM TO OR EXCEED THE MECHANICAL REQUIREMENTS OF ASTM A307. THE USE OF THIS ALTERNATE SHALL BE APPROVED BY THE ENGINEER.

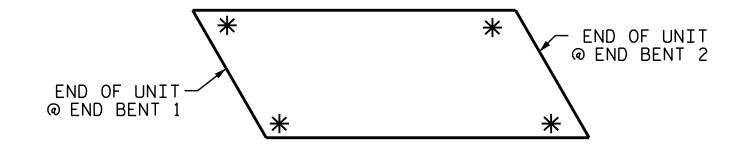
THE GUARDRAIL ANCHOR ASSEMBLY IS REQUIRED AT ALL POINTS WHERE APPROACH GUARDRAIL IS TO BE ATTACHED TO THE END OF THE PARAPET. FOR POINTS OF ATTACHMENT, SEE SKETCH.

AFTER INSTALLATION, THE EXPOSED THREAD OF THE BOLT SHALL BE BURRED WITH A SHARP POINTED TOOL.

THE COST OF THE GUARDRAIL ANCHOR ASSEMBLIES WITH BOLTS, NUTS AND WASHERS COMPLETE IN PLACE, SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS.

THE VERTICAL REINFORCING BARS MAY BE SHIFTED SLIGHTLY IN THE END POST TO CLEAR ASSEMBLY BOLTS.

THE 1 $\frac{1}{4}$ " Ø HOLES SHALL BE FORMED OR DRILLED WITH A CORE BIT. IMPACT TOOLS WILL NOT BE PERMITTED. ANY CONCRETE DAMAGED BY THIS WORK SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.



SKETCH SHOWING POINTS OF ATTACHMENT

*LOCATION OF GUARDRAIL ATTACHMENT

PROJECT NO. <u>DF18311.2095167.PR</u> WATAUGA _ COUNTY

10+92.00-L-STATION:_

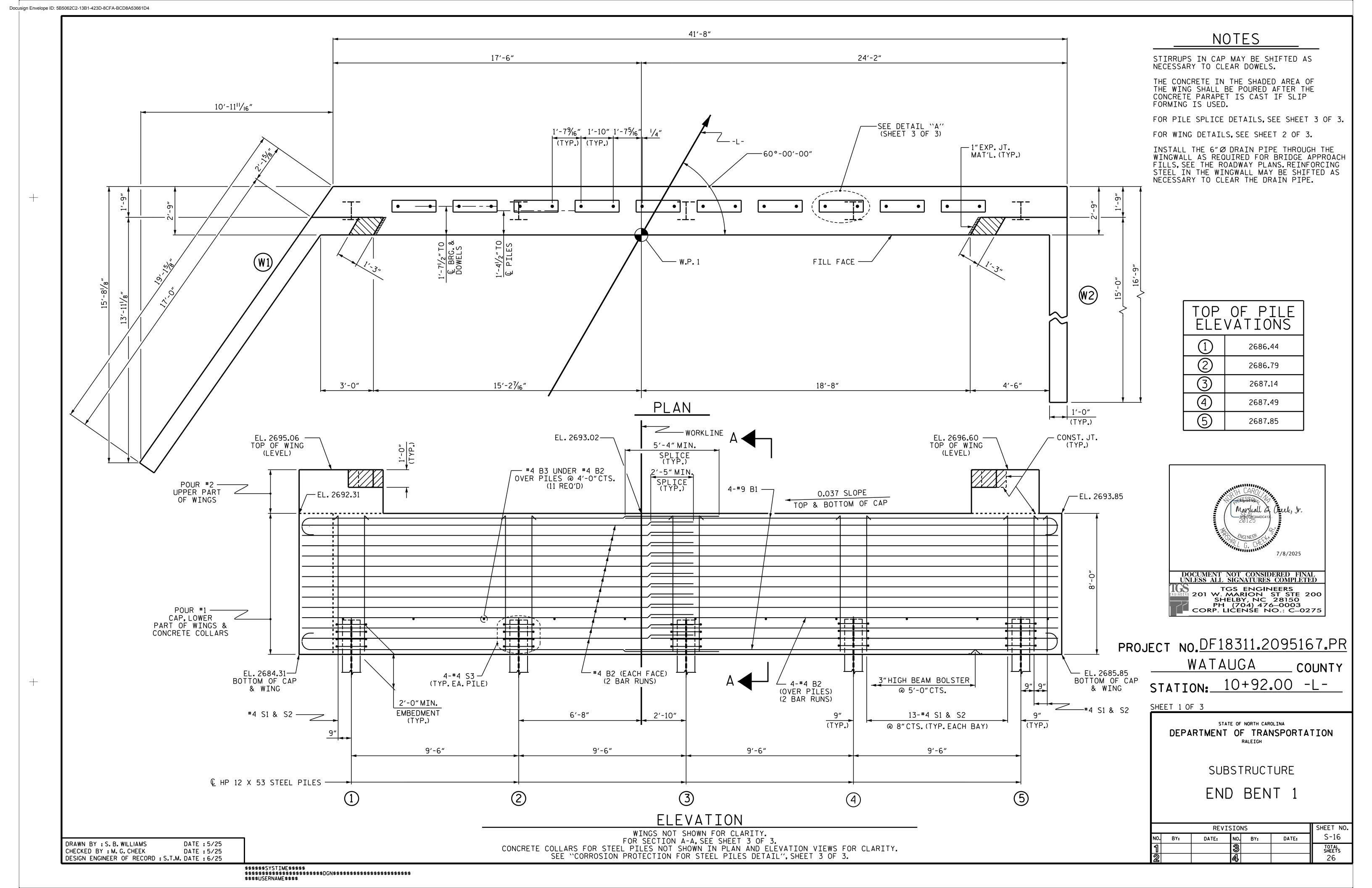


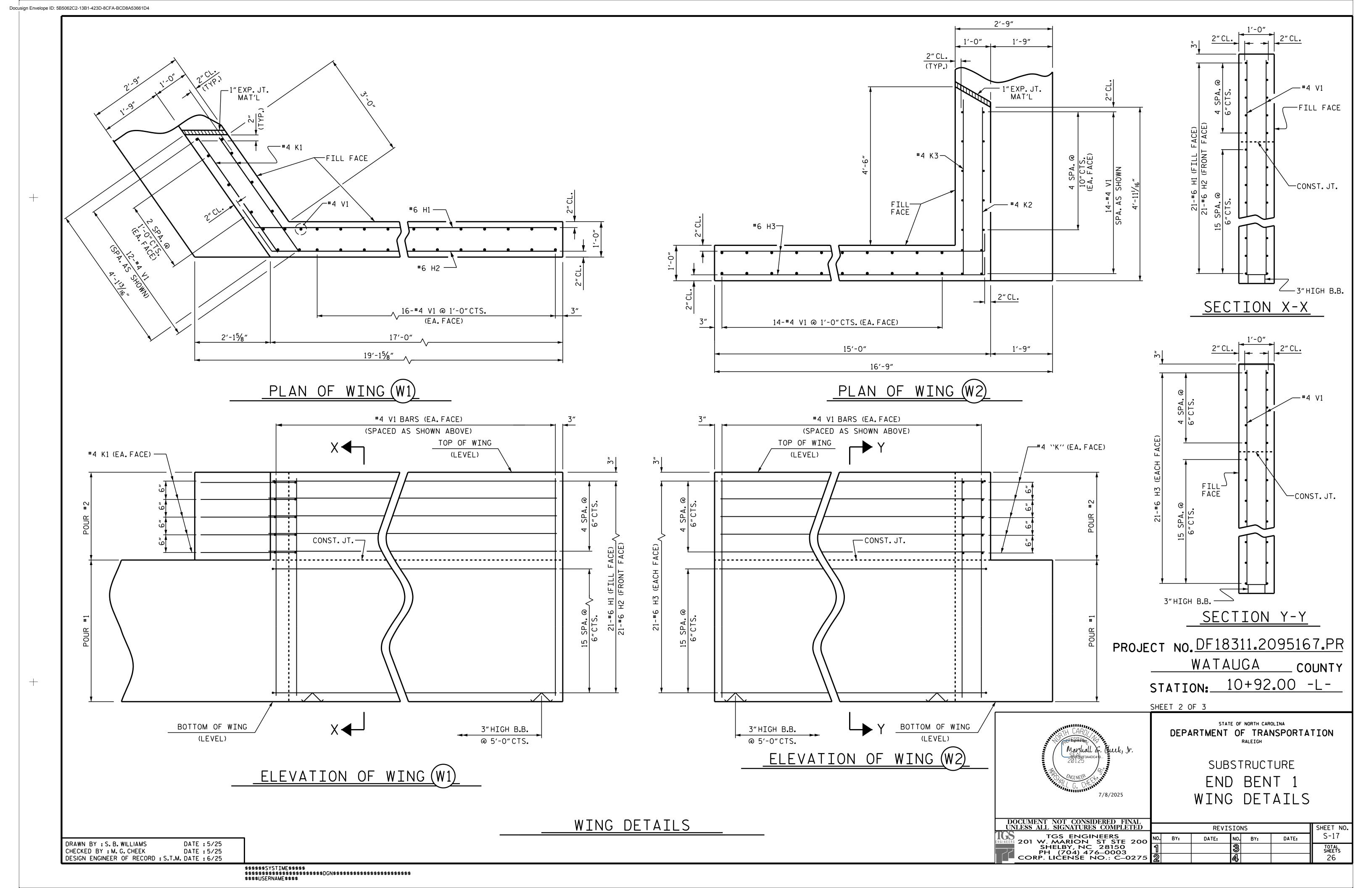
STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH

STANDARD GUARDRAIL ANCHORAGE DETAILS FOR METAL RAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED

SHEET NO. REVISIONS TGS ENGINEERS
201 W. MARION ST STE 200
SHELBY, NC 28150
PH (704) 476–0003
CORP. LICENSE NO.: C-0275 S-15 NO. BY: DATE: DATE: BY: TOTAL SHEETS 26





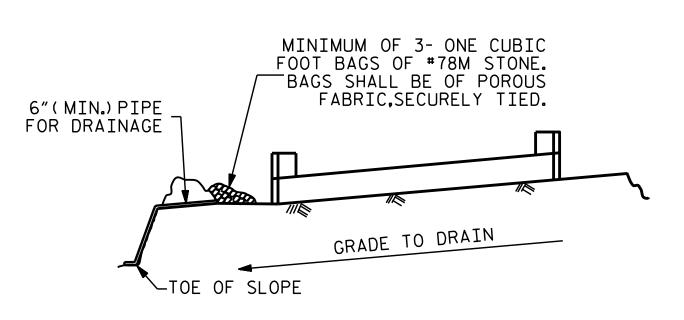
DRAWN BY : S. B. WILLIAMS

CHECKED BY : M. G. CHEEK

DESIGN ENGINEER OF RECORD : S.T.M. DATE : 6/25

DATE : 5/25

DATE : 5/25

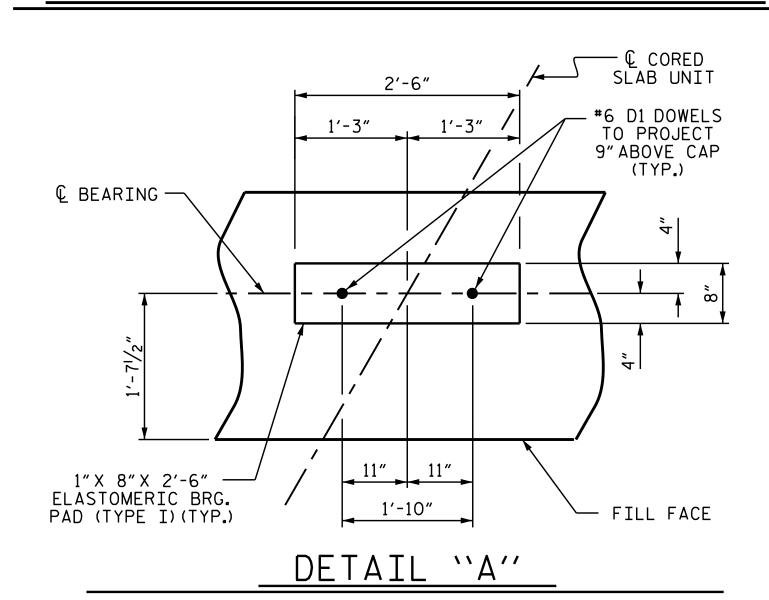


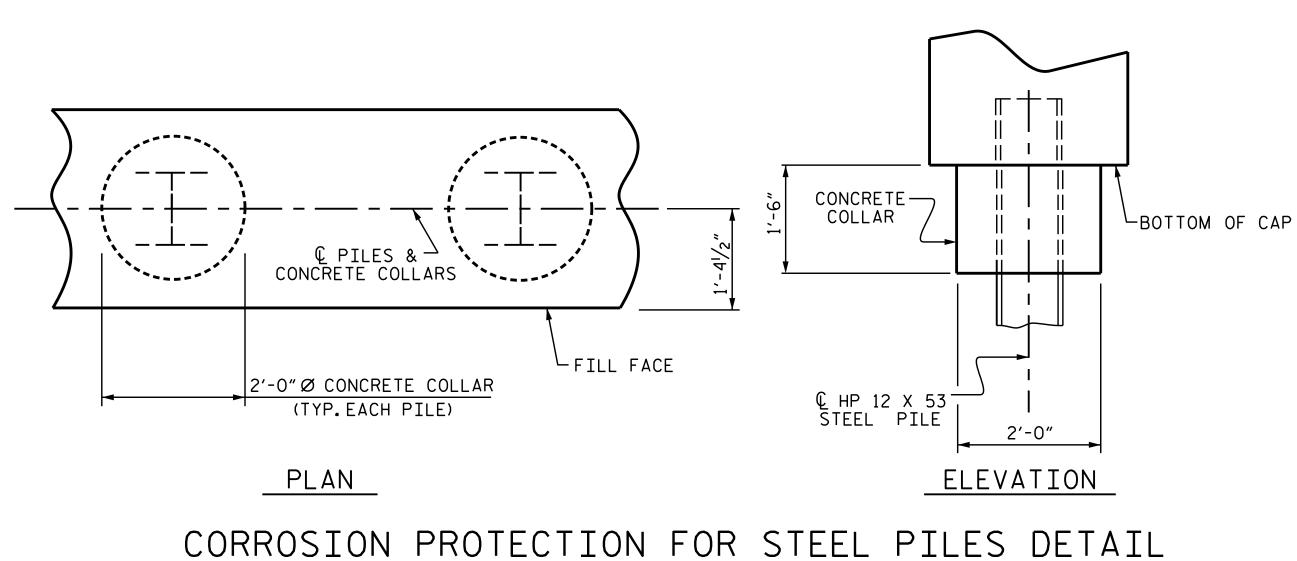
BAGGED STONE AND PIPE SHALL BE PLACED IMMEDIATELY AFTER COMPLETION OF END BENT EXCAVATION. PIPE MAY BE EITHER CONCRETE, CORRUGATED STEEL, CORRUGATED ALUMINUM ALLOY, OR CORRUGATED PLASTIC. PERFORATED PIPE WILL NOT BE ALLOWED.

BAGGED STONE SHALL REMAIN IN PLACE UNTIL THE ENGINEER DIRECTS THAT IT BE REMOVED. THE CONTRACTOR SHALL REMOVE AND DISPOSE OF SILT ACCUMULATIONS AT BAGGED STONE WHEN SO DIRECTED BY THE ENGINEER. BAGS SHALL BE REMOVED AND REPLACED WHENEVER THE ENGINEER DETER-MINES THAT THEY HAVE DETERIORATED AND LOST THEIR EFFECTIVENESS.

NO SEPARATE PAYMENT WILL BE MADE FOR THIS WORK AND THE ENTIRE COST OF THIS WORK SHALL BE INCLUDED IN THE UNIT CONTRACT PRICE BID FOR THE SEVERAL PAY ITEMS.

TEMPORARY DRAINAGE AT END BENT





/BACK GOUGE BACK GOUGE DETAIL B BACK GOUGE' PILE VERTICAL PILE HORIZONTAL OR VERTICAL **√**.**T** 0" TO 1/8" 0" TO 1/8" DETAIL A DETAIL B POSITION OF PILE DURING WELDING.

PILE SPLICE DETAILS

BAR TYPES 24'-3" 17'-3" 16'-9" 14'-8" -1'-3" LAP <u>2'</u>-5" 6 1'-8" Ø ALL BAR DIMENSIONS ARE OUT TO OUT.

BILL OF MATERIAL FOR END BENT BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 25′-6″ #9 1387 B2 56 #4 STR 22'-10" #4 STR 2'-5" B3 11 18 D1 20 #6 | STR | 1'-6" 45 565 H1 | 21 | **#**6 | 2 | 17'-11" **#**6 2 17′-5″ H2 21 549 H3 42 #6 3 | 15'-4" 967 #4 | STR | 3'-9" 25 K1 | 10 K2 5 #4 | STR | 4'-8" 16 K3 | 5 #4 | STR | 4'-11" 16 18'-5" S1 | 55 #4 677 4 | S2 55 3'-2" #4 5 116 S3 20 6'-6" 87 #4 6 V1 | 87 | #4 | STR | 10'-5" 605 5927 LBS. REINFORCING STEEL

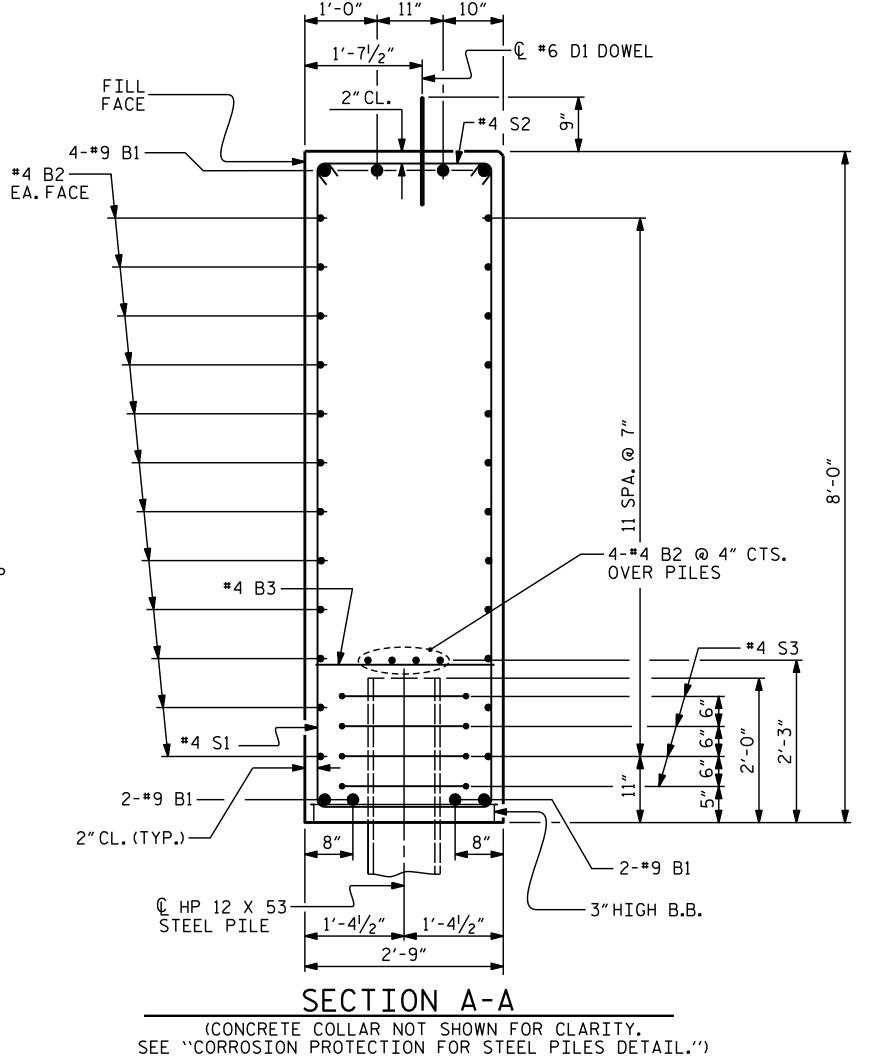
CLASS A CONCRETE BREAKDOWN

POUR #1 CAP, LOWER PART 44.5 C.Y. OF WINGS & COLLARS

4.0 C.Y.

POUR #2 UPPER PART OF WINGS

TOTAL CLASS A CONCRETE 48.5 C.Y.



PROJECT NO.DF18311.2095167.PR

WATAUGA __ COUNTY

STATION: 10+92.00 -L-

SHEET 3 OF 3

Marshall & SEBC 253A4DC41 7/8/2025

RALEIGH

SUBSTRUCTURE

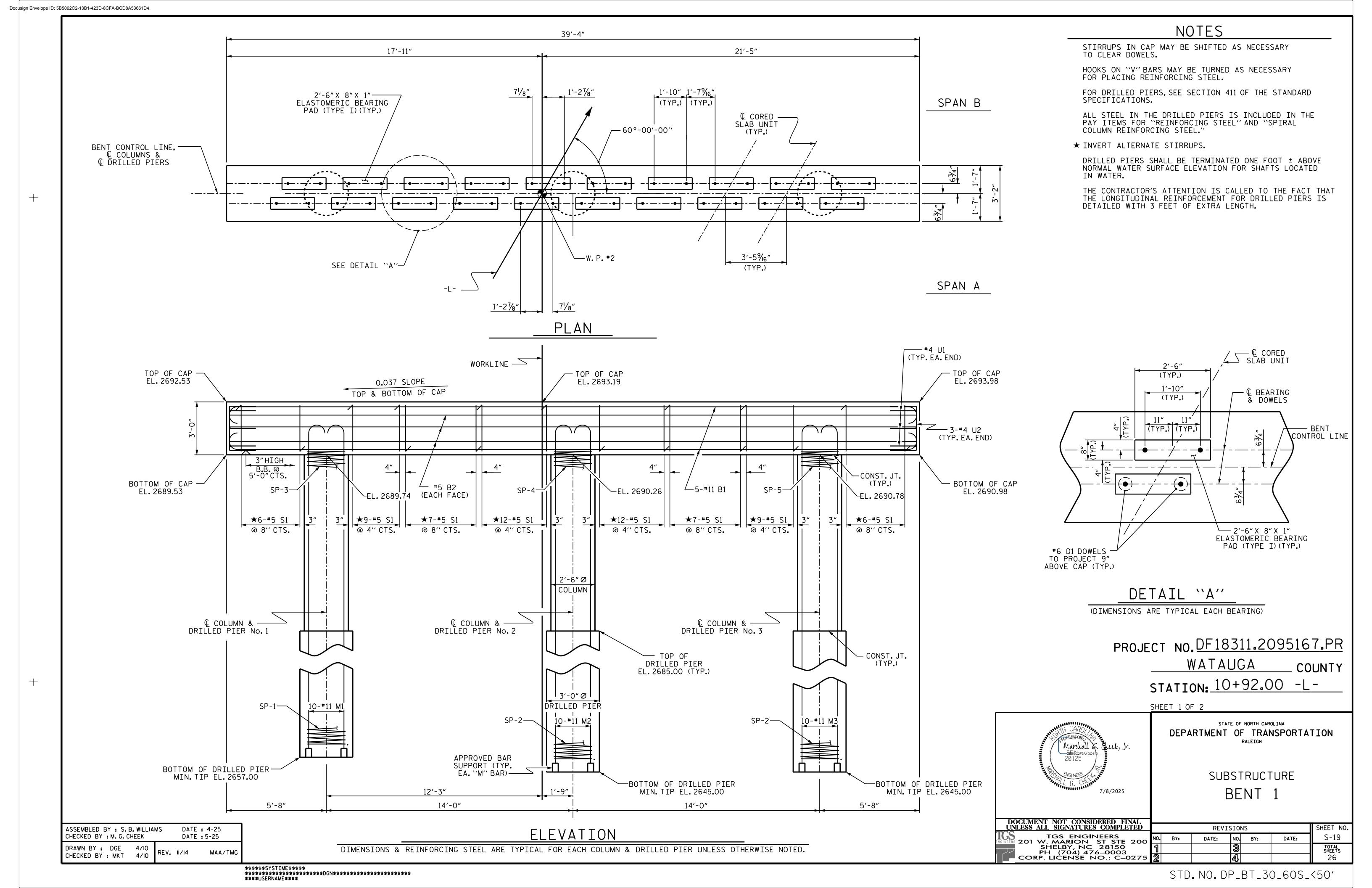
STATE OF NORTH CAROLINA

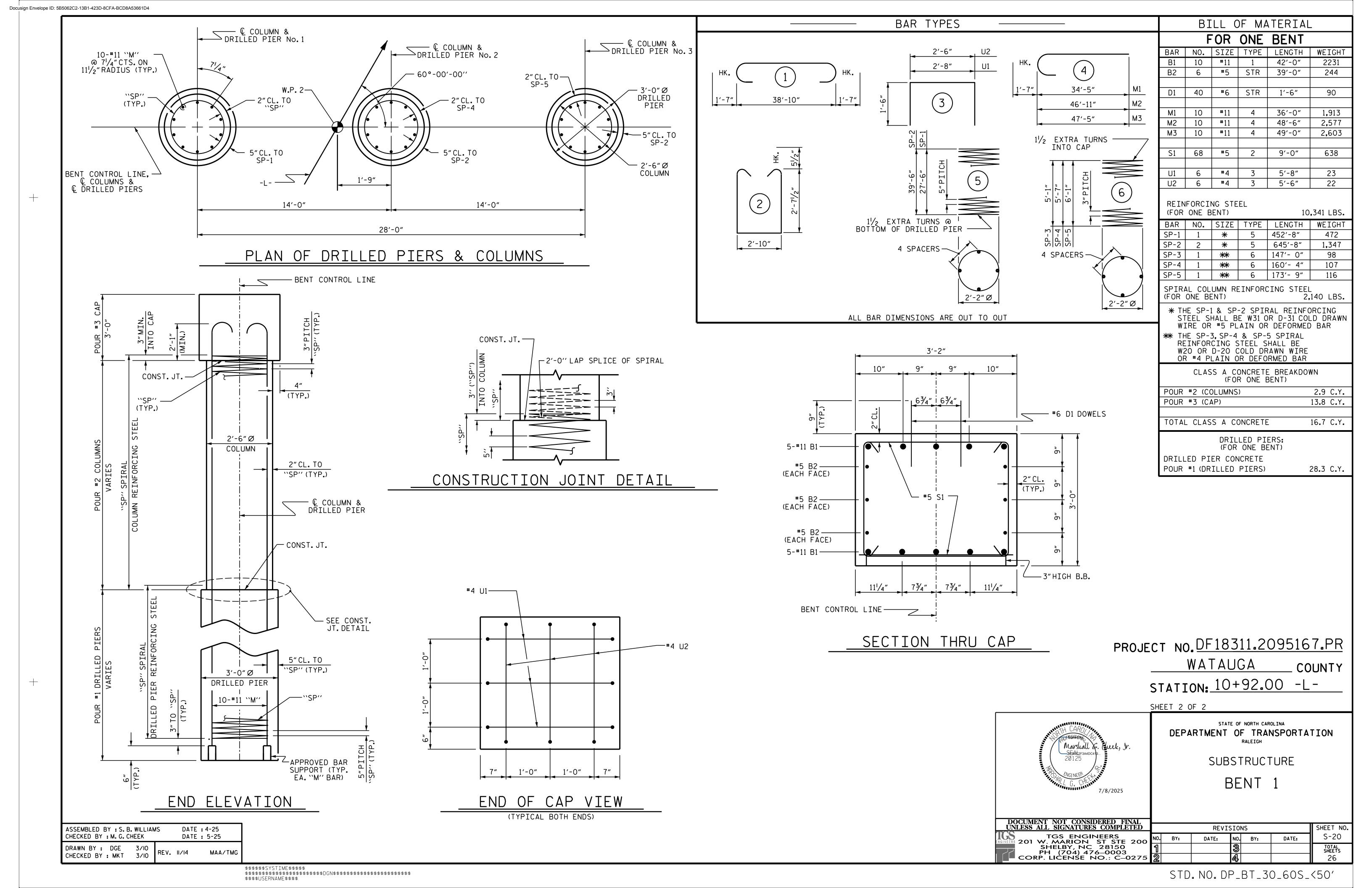
DEPARTMENT OF TRANSPORTATION

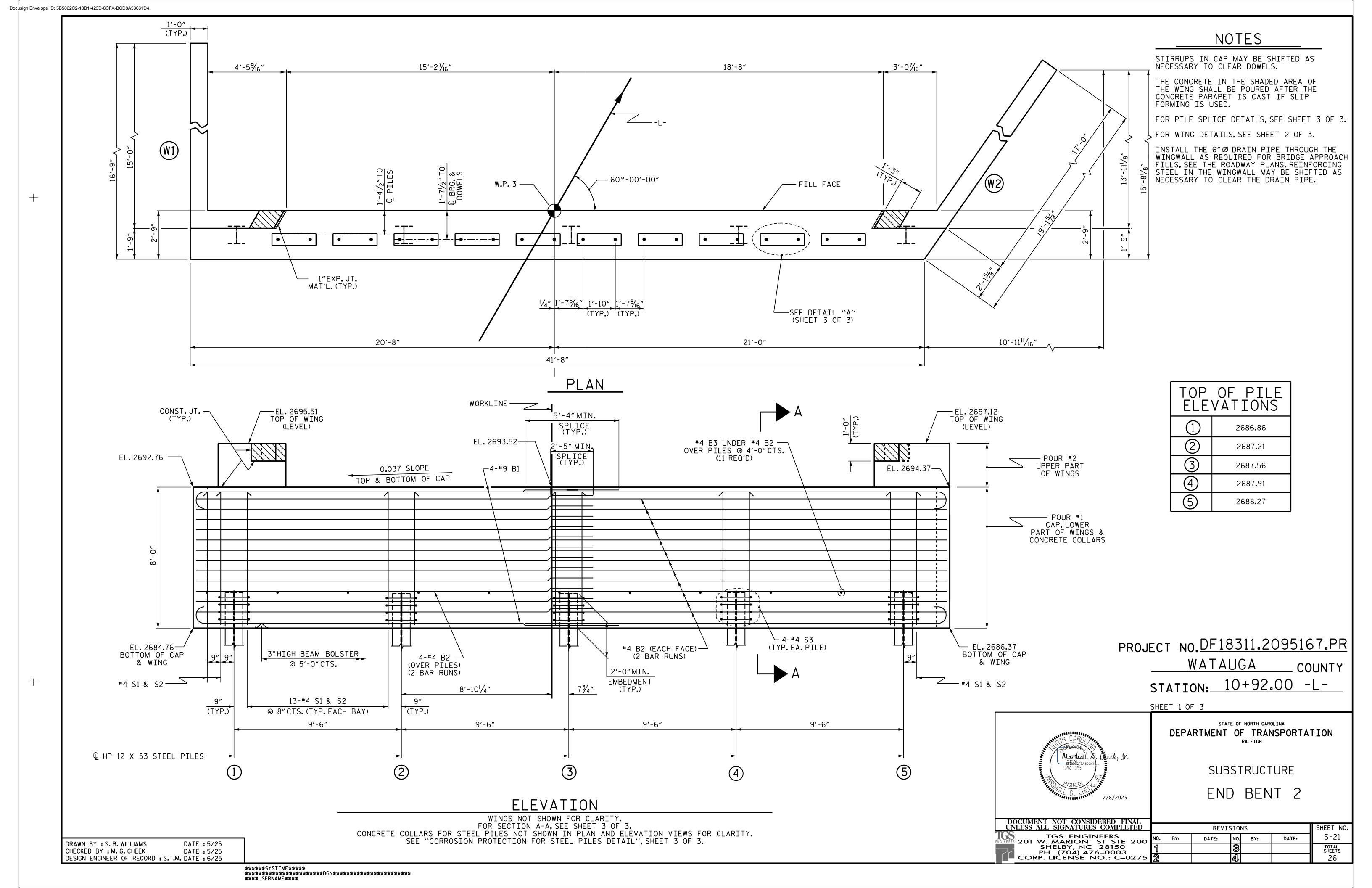
END BENT 1 DETAILS

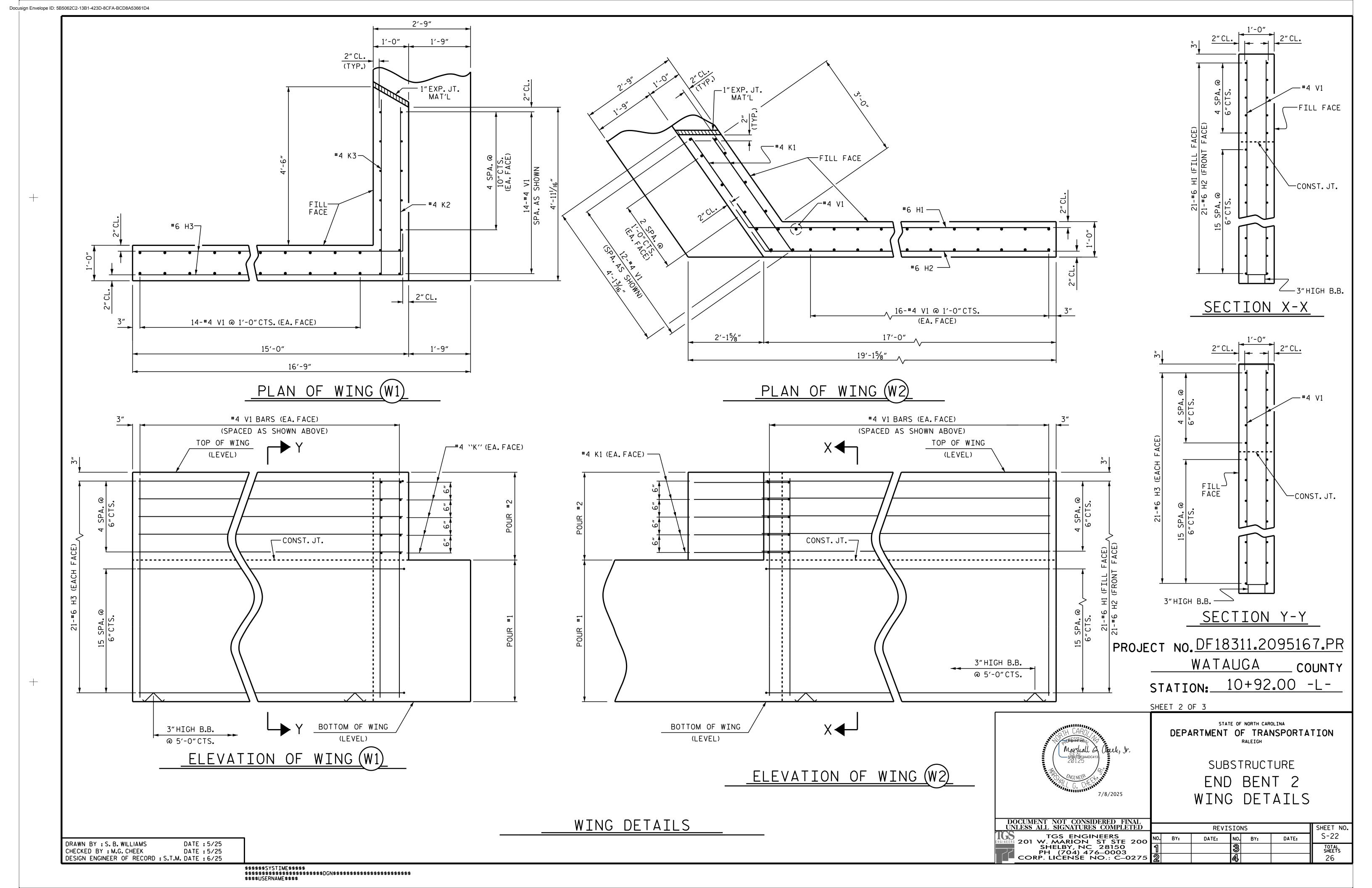
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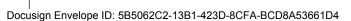
SHEET NO. **REVISIONS** S-18 NO. BY: DATE: DATE: BY: TOTAL SHEETS 26









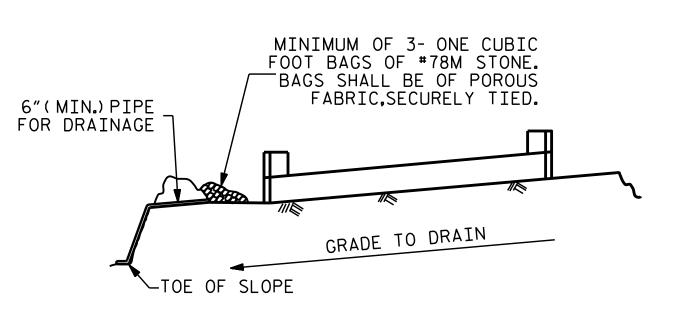


DRAWN BY : S. B. WILLIAMS

CHECKED BY: M. G. CHEEK DATE: 5/25
DESIGN ENGINEER OF RECORD: S.T.M. DATE: 6/25

DATE : 5/25

DATE : 5/25

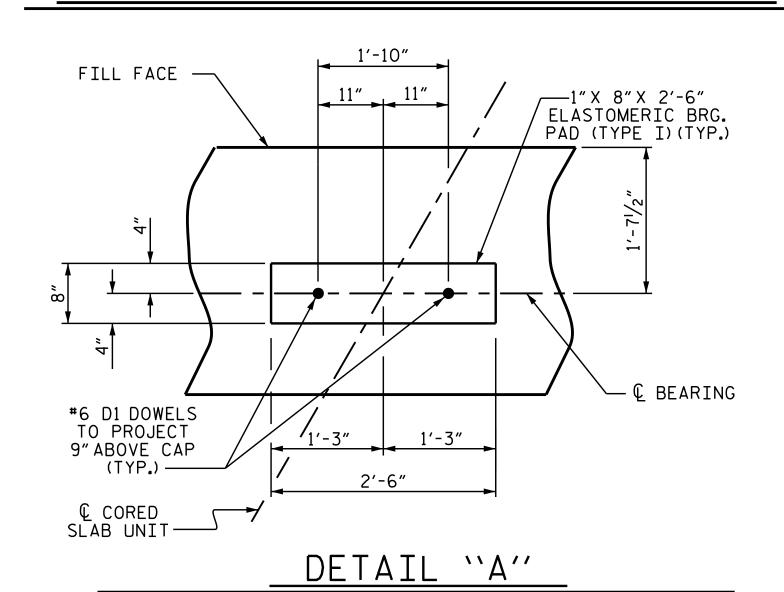


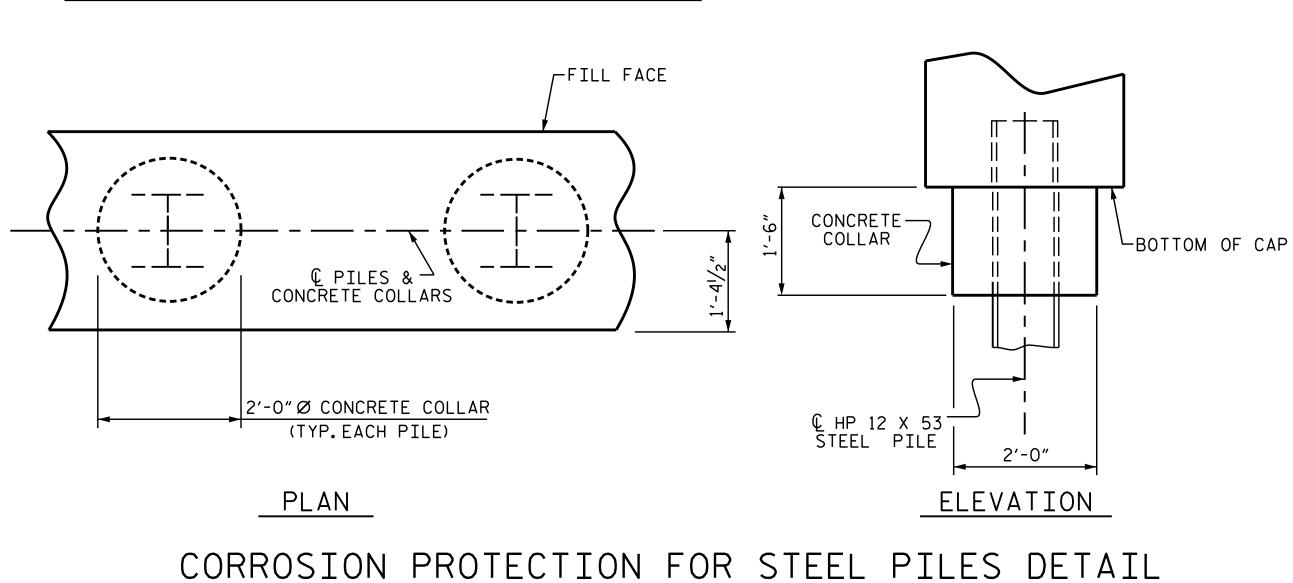
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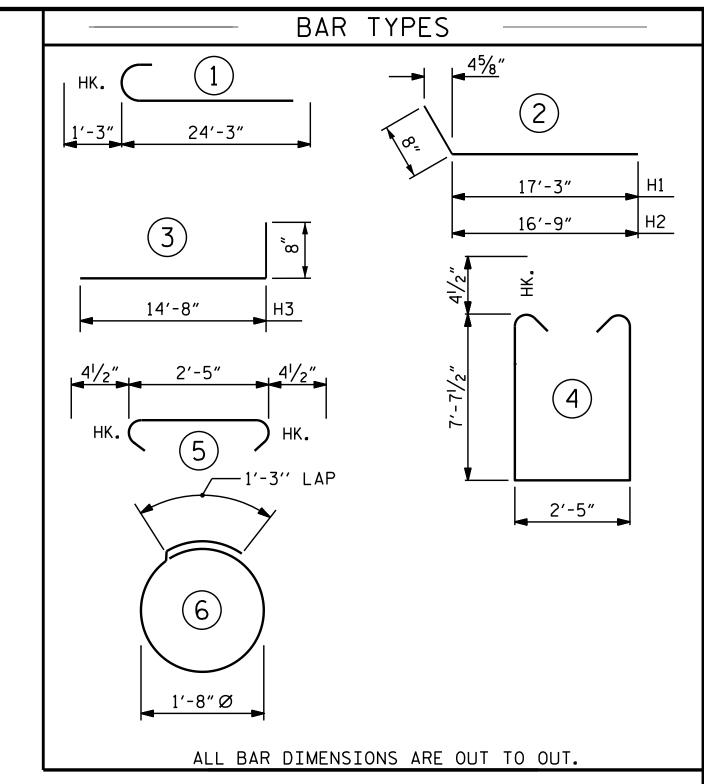
TEMPORARY DRAINAGE AT END BENT





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PILE SPLICE DETAILS



BILL OF MATERIAL FOR END BENT 2 BAR | NO. | SIZE | TYPE | LENGTH | WEIGHT 25'-6" #9 1387 #4 STR 22'-10" B2 56 #4 STR 2'-5" B3 11 18 D1 20 #6 | STR | 1'-6" 45 565 H1 | 21 | **#**6 | 2 | 17'-11" H2 21 **#**6 2 17′-5″ 549 H3 42 #6 3 | 15'-4" 967 #4 | STR | 3'-9" 25 K1 | 10 K2 5 #4 | STR | 4'-8" 16 K3 | 5 #4 | STR | 4'-11" 16 18'-5" S1 | 55 #4 677 4 | S2 55 3'-2" #4 5 116 S3 20 6'-6" 87 #4 6 V1 | 87 | #4 | STR | 10'-5" 605 5927 LBS. REINFORCING STEEL

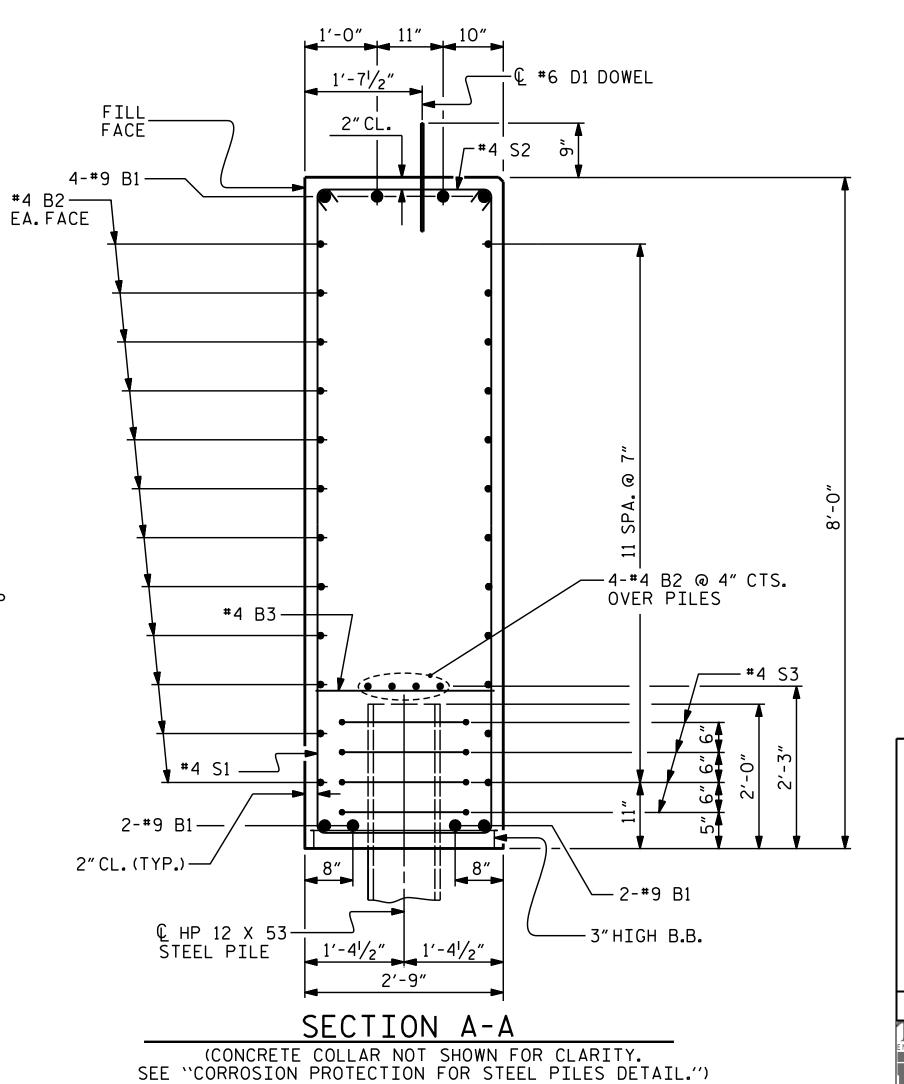
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PROJECT NO.DF18311.2095167.PR

WATAUGA __ COUNTY

STATION: 10+92.00 -L-

SHEET 3 OF 3

Marshall

DEPARTMENT OF TRANSPORTATION RALEIGH

SUBSTRUCTURE

STATE OF NORTH CAROLINA

END BENT 2 DETAILS

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED TGS ENGINEERS 201 W. MARION ST STE 200 SHELBY, NC 28150 PH (704) 476–0003 CORP. LICENSE NO.: C–0275

SHEET NO **REVISIONS** S-23 NO. BY: DATE: DATE: BY: TOTAL SHEETS

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SHOULDER LINE —

EL. 2692.10

END BENT 1

EL. 2692.10

SHOULDER LINE C

ZCS MGC

ASSEMBLED BY : CHECKED BY :

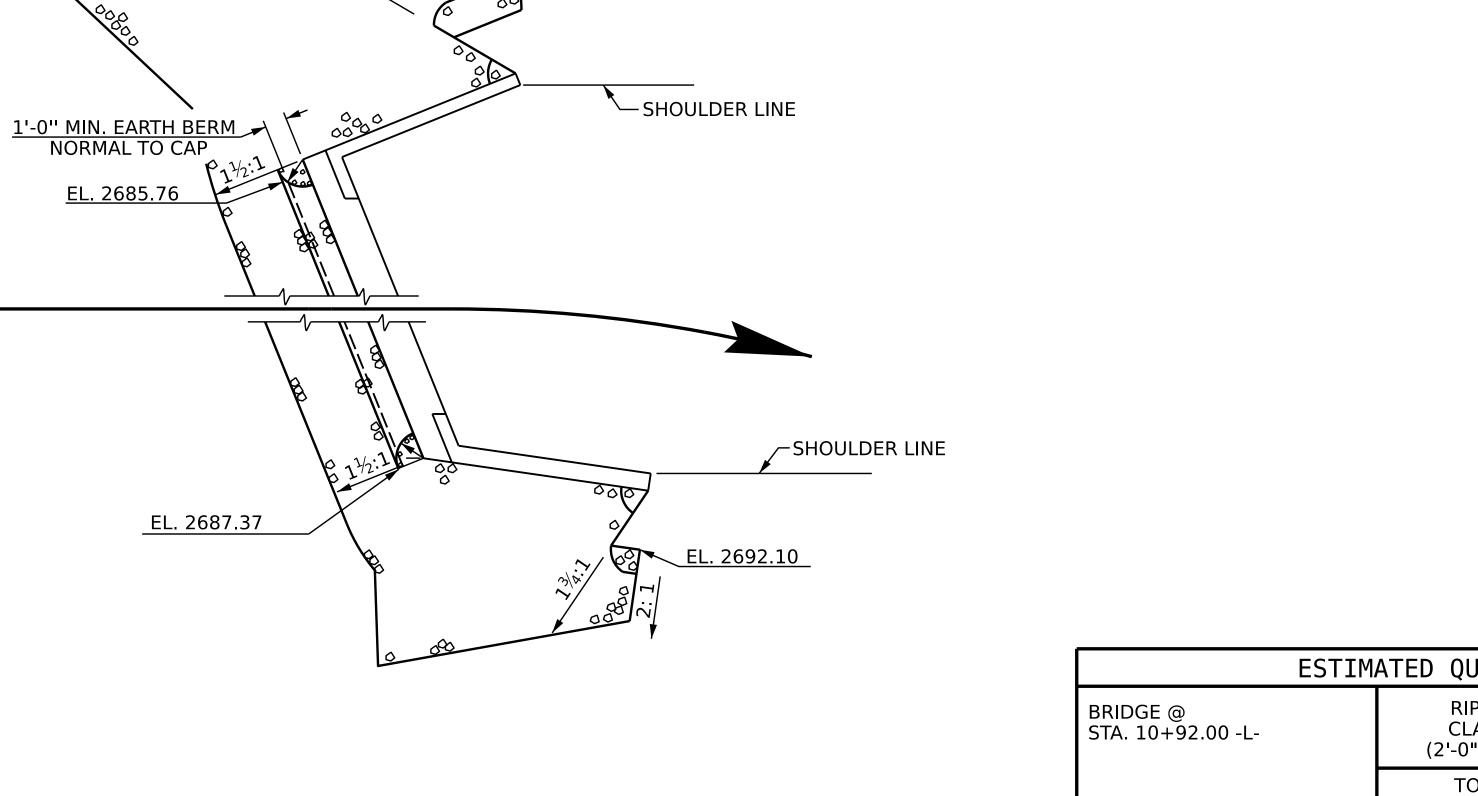
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MAA/GM MAA/GM MAA/THC

REV. 10/1/11 REV. 12/21/11 REV. 12/17



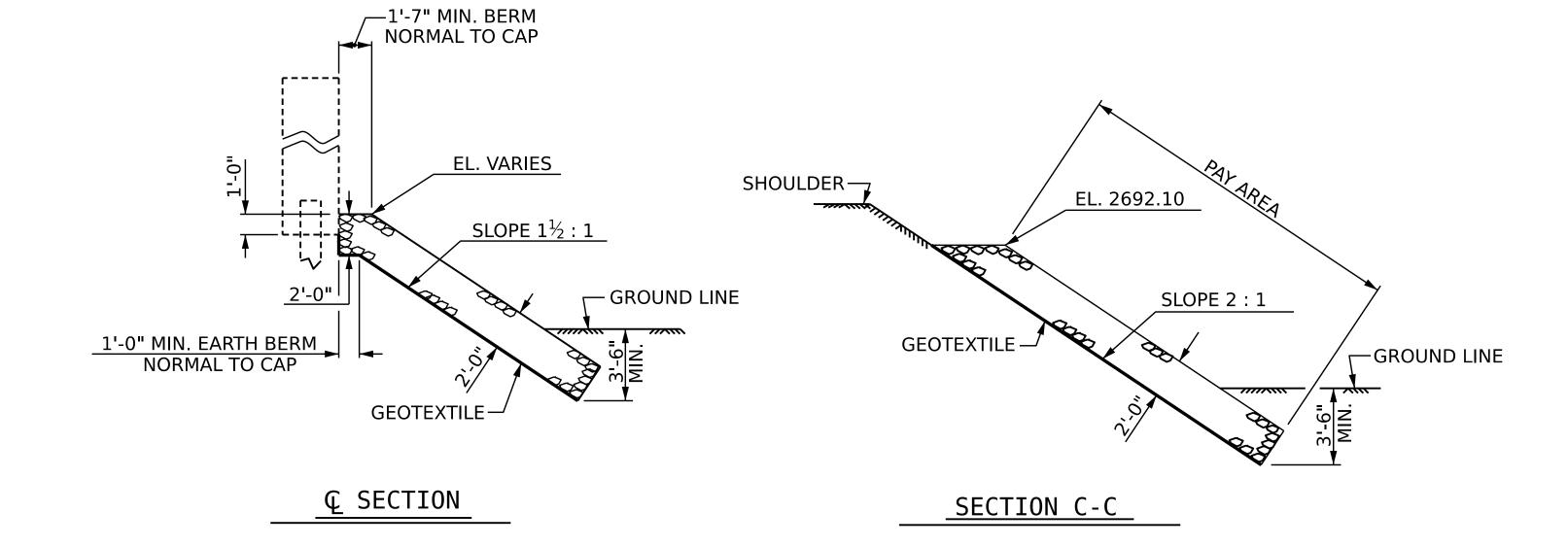
FOR BERM WIDTH DIMENSIONS, SEE GENERAL DRAWING.



EL. 2692.10

END BENT 2

ESTIMATED QUANTITIES			
BRIDGE @ STA. 10+92.00 -L-	RIP RAP CLASS II (2'-0" THICK)	GEOTEXTILE FOR DRAINAGE	
	TONS	SQUARE YARDS	
END BENT 1	155	175	
END BENT 2	175	195	



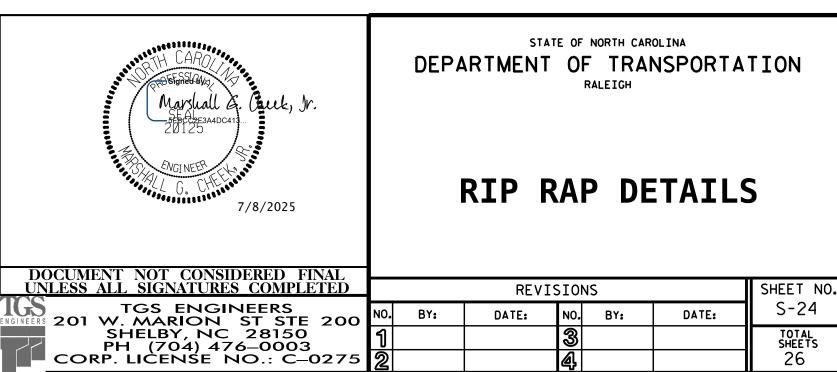
<u>PLAN</u>

BERM RIP RAPPED

PROJECT NO. DF18311.2095167.PR

WATAUGA county

STATION: 10+92.00 -L-



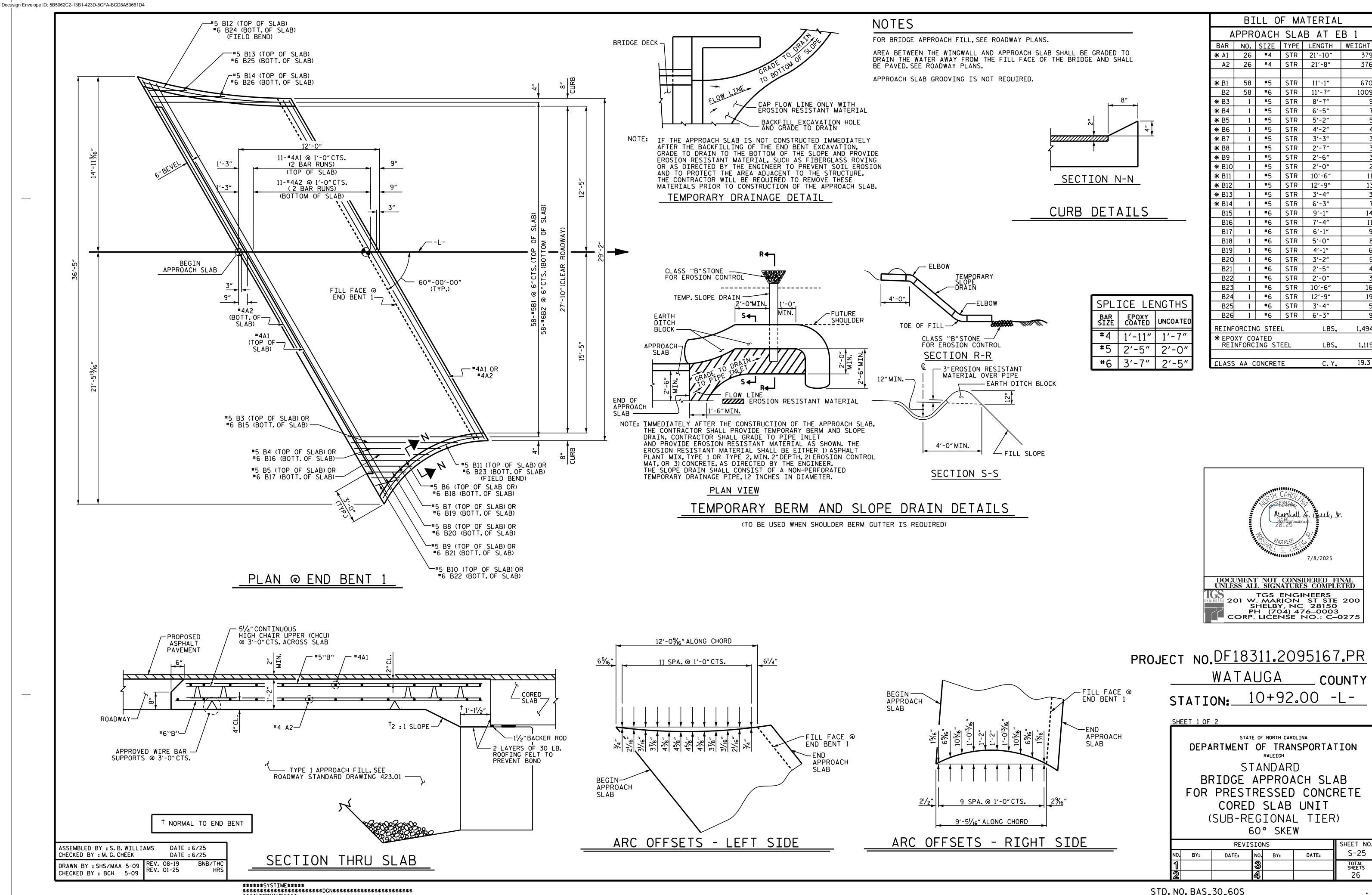
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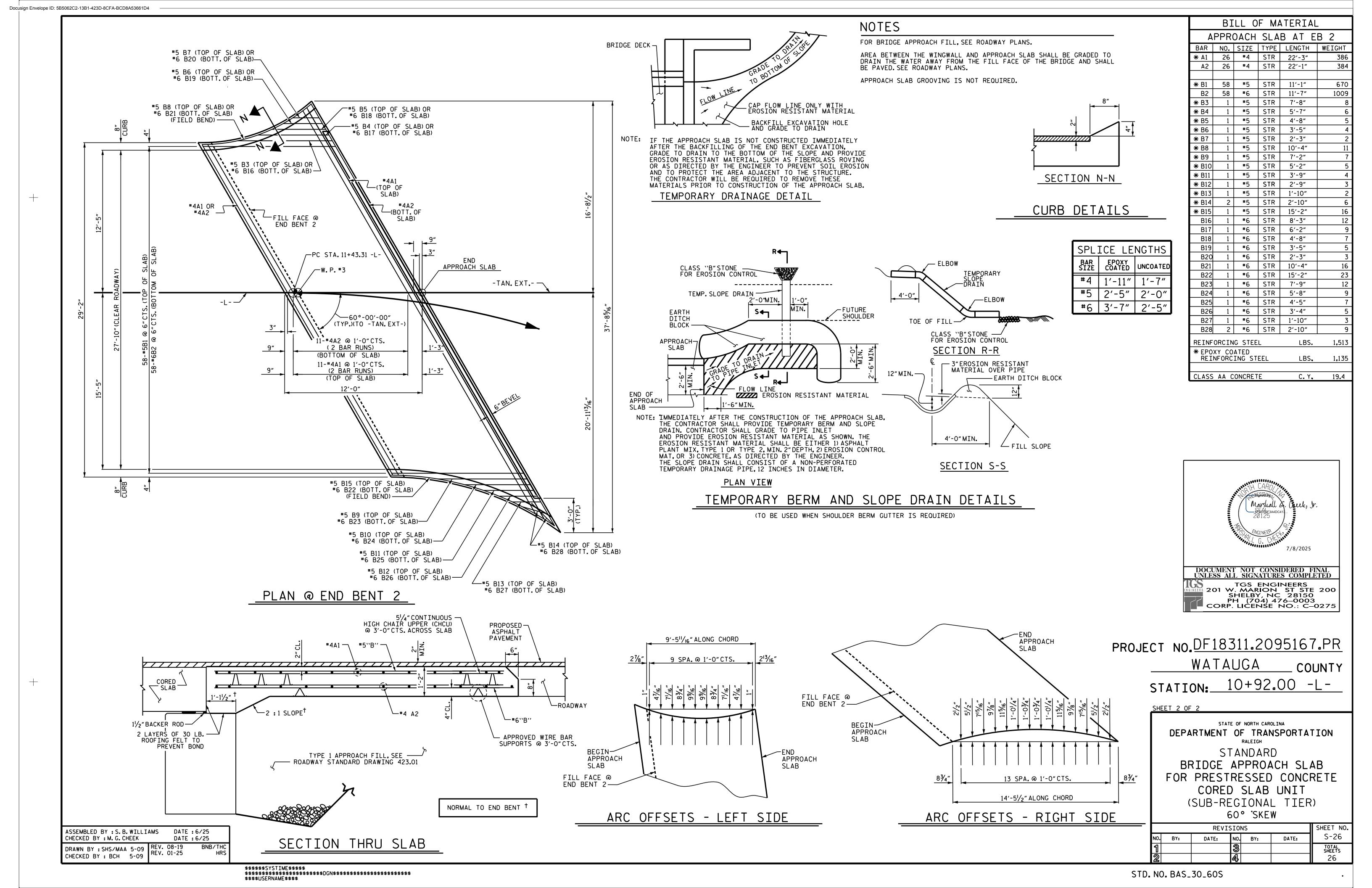
EL. 2685.31

EL. 2686.85

1'-0" MIN. EARTH BERM NORMAL TO CAP

STD. NO. RR1





STANDARD NOTES

DESIGN DATA:

SPECIFICATIONS.... AASHTO (CURRENT) LIVE LOAD SEE PLANS IMPACT ALLOWANCE..... SEE AASHTO STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36 ____ 20,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50W ___ 27,000 LBS. PER SQ. IN. - AASHTO M270 GRADE 50 ____ 27,000 LBS, PER SQ, IN. REINFORCING STEEL IN TENSION - GRADE 60 _____ 24,000 LBS. PER SQ. IN. CONCRETE IN COMPRESSION _______ 1,200 LBS. PER SQ. IN. CONCRETE IN SHEAR SEE AASHTO STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS ____ 1,800 LBS. PER SQ. IN. COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER _____ 375 LBS. PER SQ. IN. EQUIVALENT FLUID PRESSURE OF EARTH ______ 30 LBS. PER CU. FT.

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2024 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N. C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO $\frac{4}{2}$ " RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A $\frac{1}{4}$ " FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A $\frac{1}{4}$ " RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS. CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF $3 - \frac{7}{8}$ " Ø STUDS FOR $4 - \frac{3}{4}$ " Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ " Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ " Ø STUDS BASED ON THE RATIO OF $3 - \frac{7}{8}$ " Ø STUDS FOR $4 - \frac{3}{4}$ " Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-0".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2" OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY $\frac{1}{16}$ " OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.